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Fracture Mechanics Analysis of Centered & Offset Fastener Holes in Stiffened & Unstiffened Panels under Uniform Tension

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FRACTURE MECHANICS ANALYSIS OF CENTERED AND OFFSET FASTENER HOLES IN STIFFENED AND UNSTIFFENED PANELS UNDER UNIFORM TENSION

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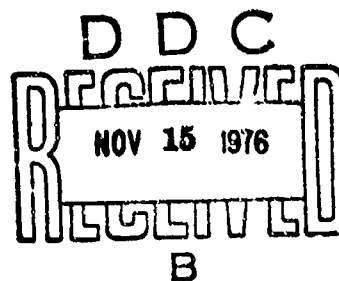
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hole is presented. A detailed description of the programming covers the physical problem, modelling, program flow and options, input/output conventions, execution times, and limitations which must be observed. Results are presented for performance tests of the special element, and for some example analyses of cracked panels.

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FOREWORD

The developments documented in this report were carried out at the Aeroelastic and Structures Research Laboratory, Massachusetts Institute of Technology, Cambridge, Massachusetts 02139, under Contract No. F33615-74-C-3063 (Project 1367, Task 136703) from the U.S. Air Force Flight Dynamics Laboratory. Mr. James L. Rudd (AFFDL/FBE) served as technical monitor. The authors gratefully acknowledge the many contributions by Mrs. Susan E. French of the Aeroelastic and Structures Research Laboratory. Mrs. French was involved with the detailed programming aspects of the work throughout the project. This report is the second in a series, covering research conducted during October 1974-February 1975, and was submitted for technical review in May 1975. The other reports in this series are AFFDL-TR-75-51 (Fracture Mechanics Analysis of an Attachment Lug), AFFDL-TR 75-71, (Fracture Mechanics Analysis of Single and Double Rows of Fastener Holes Loaded in Bearing), and AFFDL-TR-76-12 (Numerical Computation of Stress Intensity Factors for Aircraft Structural Details by the Finite Element Method). The contractor's report number is ASRL TR 177-2.

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Section 1

INTRODUCTION

This is the second of a series of reports on the development of finite-element analysis procedures for computation of linear elastic stress intensity factors associated with cracks in common aircraft structural details. Finite-element analysis serves the important purpose of providing K_I and K_{II} solutions for situations in which the boundary geometries are too complicated to permit convenient treatment by the classical methods.

The first report in the series [1] summarized the formulation of two basic building blocks which are used in the analyses: the well-known bilinear isoparametric quadrilateral and the PCRK59 assumed-stress hybrid crack-containing elements. The former element has been widely accepted and appears in most general-purpose finite-element programs. The latter element was originally developed at ASRI in 1972 [2], and has subsequently been put through extensive performance tests to characterize its behavior as a function of shape distortions [3].

Earlier work [4, 5] has demonstrated that the assumed-stress hybrid method permits accurate computation of stress intensity factors, while preserving Matrix Displacement Method conventions in the global programming environment (data input, assembly, matrix solution procedure, etc.). In the present work, the hybrid method is used again to formulate another special-

purpose element for modelling the region around a fastener hole. The new element is combined with the quadrilateral and PCRK59 to represent a skin tension panel containing an open fastener hole. The ASRL FEABL program [6] is again used for global analysis. Additional features belonging to FEABL which have not yet been formally documented are employed in the present analysis, and are outlined briefly in this report. These features pertain to the technique of substructuring by a Gauss elimination algorithm.

Section 2

NEW DEVELOPMENTS

Modularization of the finite-element model of a structure becomes as important as modularization of the basic program when the application is computation of stress intensity factors. The objective is to reduce unnecessary arithmetic operations and computer printout to a minimum. In particular, the stresses and displacements are of little interest (and of no interest at all at any distance away from the crack tip) except for verifications of accuracy during development. The substructuring technique, used for many years in airframe analysis programs*, provides the required modularity. Subroutines for substructuring were developed as add-ons to FEABL as part of an unrelated project** and have been used in the present work. The substructuring algorithm and the functions and conventions of the add-on subroutines are discussed briefly in this section to provide a complete picture of the panel analysis program.

The so-called "near-field" region around a fastener hole forms one such substructure module. The first attempt to model this region with conventional quadrilateral elements resulted in

*For example, the Boeing ASTRA program.

**Sponsored by the Xerox Corporation (Dr. T. C. Soong, technical monitor).

severe mesh distortion, numerous unwanted degrees of freedom, and solution accuracy problems. In consequence, the hybrid method was called upon, resulting in the HOLEI special-purpose element for this module. The history of these investigations is traced in Subsections 2.3 and 2.4.

2.1 Substructuring

Figure 1 illustrates the finite-element model of a square panel which contains a single fastener hole offset from the panel centerline. A detail in the lower part of the figure shows how a PCRK59 element might be inserted to represent a small crack emanating from the hole. It is evident that the complete global solution (displacements at each of the many nodes in the model) constitutes unwanted information if the analysis is to be repeated while the crack location is varied parametrically around the fastener hole.

A computationally more efficient procedure is obtained by subdividing the model into far-field and near-field substructures, as shown in Fig. 2. The broken line in Fig. 2 corresponds to the edges ABCD in Fig. 1. If the element stiffnesses, prescribed nodal forces and nodal displacements are first assembled for the far-field substructure, the resulting equation system may be partially solved. In effect, all of the information pertaining to the far field substructure is transferred to the inter-substructure boundary ABCD. This information may then be treated as a "super-element", which can be assembled with a succession

of near-field substructures containing a crack in different locations. As a result, many fewer degrees of freedom are processed in the parametric stress intensity computations.

The substructuring process may be represented formally as follows. Let the assembled equation system $\underline{K}\underline{q} = \underline{\hat{Q}}$ for a substructure be partitioned into:

$$\begin{bmatrix} \underline{K}_{II} & \underline{K}_{IB} \\ \underline{K}_{BI} & \underline{K}_{BB} \end{bmatrix} \begin{bmatrix} \underline{q}_I \\ \underline{q}_B \end{bmatrix} = \begin{bmatrix} \underline{\hat{Q}}_I \\ \underline{\hat{Q}}_B \end{bmatrix} \quad (1)$$

where

\underline{K} = stiffness coefficients
 \underline{q} = (unknown) nodal displacements
 $\underline{\hat{Q}}$ = prescribed nodal forces

and where $\underline{K}_{BI} = \underline{K}_{IB}^T$. Subscripts I and B refer to nodes and degrees of freedom which are considered respectively as "interior" (to be eliminated) and "boundary" (to be retained for subsequent assembly along the inter-substructure boundary). The interior degrees of freedom are formally eliminated by solving the first of Eqs. 1 and substituting into the second:

$$\begin{aligned} \underline{K}_{II} \underline{q}_I + \underline{K}_{IB} \underline{q}_B &= \underline{\hat{Q}}_I \\ \underline{q}_I &= \underline{K}_{II}^{-1} \underline{\hat{Q}}_I - \underline{K}_{II}^{-1} \underline{K}_{IB} \underline{q}_B \end{aligned} \quad (2)$$

$$\underline{K}_{BI} (\underline{K}_{II}^{-1} \underline{\hat{Q}}_I - \underline{K}_{II}^{-1} \underline{K}_{IB} \underline{q}_B) + \underline{K}_{BB} \underline{q}_B = \underline{\hat{Q}}_B \quad (3)$$

A rearrangement of the terms in Eq. 3 leads immediately to:

$$\tilde{K}_{BB}^{(c)} \tilde{q}_B = \hat{Q}_B^{(c)} \quad (4)$$

where $\tilde{K}_{BB}^{(c)}$, $\hat{Q}_B^{(c)}$ are the statically condensed stiffnesses and forces, given by:

$$\tilde{K}_{BB}^{(c)} = \tilde{K}_{BB} - \tilde{K}_{BI} \tilde{K}_{II}^{-1} \tilde{K}_{IB} \quad (5)$$

$$\hat{Q}_B^{(c)} = \tilde{Q}_B - \tilde{K}_{BI} \tilde{K}_{II}^{-1} \hat{Q}_I \quad (6)$$

It is apparent from Eqs. 5 and 6 that the substructuring process may be realized as a computing algorithm independent of the unknowns q_I , q_B . In practice, the process may be programmed to eliminate one degree of freedom (one equation) from the system at a time, so that inversion of \tilde{K}_{II} is avoided. The entire algorithm then consists of as many passes through the assembled equations as there are degrees of freedom to be eliminated, a procedure quite similar to the Gauss-Cholesky factoring methods normally used to solve the whole equation system [6].

Substructuring places an additional burden on the program user when the global software stores K as a band-matrix. It can be shown by tracing the details that the elimination algorithm inflates the stiffness matrix bandwidth if q_I and q_B are arbitrarily

interspersed. However, if \underline{q}_I always appear as the first degrees of freedom in the global numbering sequence, the bandwidth of \underline{K} is not affected by elimination. This restriction has been placed on the FEABL add-on subroutines, and it requires some care in the choice of global numbering sequence and substructure boundary locations on the user's part.

The foregoing derivation of the substructuring algorithm assumes that all the prescribed quantities are nodal forces $\hat{\underline{Q}}$, while all of the displacements \underline{q} are unknowns. However, if restraints are applied to the substructure such that a subset $\hat{\underline{q}}_{I_0}$ of the interior degrees of freedom \underline{q}_I are prescribed, Eqs. 2 through 6 may still be used. The only difference is that the rows and columns of \underline{K} corresponding to $\hat{\underline{q}}_{I_0}$ are decoupled from the equation system*, while the right hand side of Eqs. 1 is replaced by:

$$\{ \hat{\underline{Q}}_I \mid \hat{\underline{Q}}_B \} = \{ \hat{\underline{Q}}_I \text{ or } \hat{\underline{q}}_{I_0} \mid \hat{\underline{Q}}_B - \underline{K}_{BI_0} \hat{\underline{q}}_{I_0} \} \quad (7)$$

2.2 Implementation of Substructuring in FEABL

Three add-on subroutines have been programmed and verified for the global computation tasks associated with substructuring. Subroutine STACON executes the Gauss elimination algorithm on a band-matrix which has been assembled and stored in accordance with

*See Ref. 6, Subsection 3.2.5.

the existing FEABL conventions. A normal MAIN program is written to control the FEABL software up to the point at which K would be factored in a conventional analysis. However, instead of factoring the instructions:

ISGN =

CALL STACON (ISGN, NID, RNAME, INAME)

is programmed, where

ISGN = Matrix condition control parameter

NID = Total number of interior degrees of freedom

RNAME, INAME = Real (floating point) and integer
(fixed point) variable names assigned
to the FEABL DATA vector

The control parameter ISGN must be set to +1, -1 or 0 before STACON is called. A value of +1 aborts the run if K is not a positive-definite matrix. A value of -1 aborts the run if K is singular. A value of 0 always results in return to the MAIN program with ISGN reset according to the actual matrix condition encountered: positive-definite (+1), non-positive but nonsingular (negative integer), or singular (0). The positive-definite option is used in conventional finite-element analyses. The nonsingular option is used in special cases, e.g., Hermann's principle for which ideally incompressible elements include volumetric constraints in the assembled equation system. The option ISGN=0 may be used generally, in programs for which the user wishes to retain control for debugging or other purposes when an error condition is encountered. Subroutine STACON carries out the elimination process in place,

as shown schematically in Fig. 3. After execution, the statically condensed force vector appears in INAME(IQ+NID) to INAME(LQ) and the statically condensed stiffness coefficients appear in INAME(J) to INAME(LK), where:

$$J=IK+INAME(IK\emptyset UNT+NID)+NID+1 \quad (8)*$$

The shaded areas shown in Fig. 3 contain the back-substitution information $K_{II}^{-1} \hat{Q}_I$ and $K_{II}^{-1} K_{IB}$, which may be used to solve for q_I (Eq. 2) after q_B has been computed.

The statically condensed stiffnesses and forces may simply be transferred to another storage area and treated thereafter as a "super-element" if desired. A general programming sequence to execute the transfer is given in Appendix A. If this is done, the substructure may later be assembled into a complete structure with subroutine ASEMBL or subroutine ASMLTV. A list of local-to-global degree-of-freedom connections must be given for the "super-element", just as is done for any ordinary element. However, while the numbering convention for an ordinary element is fixed by the manner in which the element subroutine has been programmed, the convention for a "super-element" is determined by the user's sequence of global numbering for assembly of the substructure. For example, if the far-field substructure in Fig. 2 has been

*IK \emptyset UNT is another address control parameter. See Ref. 6, subsections 2.1.3, 2.2.2 and 2.2.6.

numbered such that the boundary degrees of freedom are globally assigned in the order A+B+C+D+A, then the connections of the far-field "super-element" to the near-field portion of the model must be given in that order.

A second option available to the user is to leave the statically condensed substructure in place and to employ a second DATA vector for assembly and solution of the complete structure. Add-on subroutine ASMSUB has been programmed for this option. Subroutine ASMSUB performs a function similar to the conventional assembly routines (ASEMBL,ASMLTV), but is capable of assembling $K_{BB}^{(c)}$, $\hat{Q}_B^{(c)}$ directly from one DATA vector into another. Since two DATA vectors must be manipulated simultaneously, extra address control parameters are required. The beginning of the program might appear as follows:

```

DIMENSION RNAME1(xxx), INAME1(xxx), RNAME2(yyy), INAME2(yyy)
EQUIVALENCE (RNAME1(1), INAME1(1))
EQUIVALENCE (RNAME2(1), INAME2(1))
COMMON /SIZE/ NET, NDT
COMMON /BEGIN/ ICON, IKOUNT, ILNZ, IMASTR, IQ, IK
COMMON /END/ LCAN, LKOUNT, LLNZ, LMASTR, LQ, LK
COMMON /SIZESS/ NETSS, NDTSS, NIDSS
COMMON /BEGSS/ IBEG(6)
COMMON /ENDSS/ LEND(6)
.
.
.
```

Now suppose that a substructure is assembled and statically

condensed in vector (RNAME1, INAME1):

.
.
.

ISGN=1

CALL STACON(ISGN, NID, RNAME1, INAME1)

At this point, the substructure's control information must be cleared from the labelled COMMON areas to permit formation of the complete structure. The information is transferred to the extra areas:

NETSS=NET

NDTSS=NDT

NIDSS=NID

IBEG(1)=ICON

.
.
.

LEND(6)=LK

.
.
.

The program continues with a conventional generation of size, connections, assembly, etc. for the complete structure in vector (RNAME2, INAME2) until the substructure is to be assembled. At this point, the user simply programs:

.
.
.

CALL ASMSUB(LNUM, RNAME1, INAME1, RNAME2, INAME2)

.
 .
 .

where LNUM represents the element number assigned to the substructure, considered as a "super-element" in the complete structure.

There are some cases for which the solution q_I may be required in a particular substructure. Subroutine QBACK has been programmed as the third add-on to execute the required back-substitution solution for q_I . Subroutine QBACK is designed to work with ASMSUB in the multiple DATA vector environment. Suppose that the above example is continued to the point at which assembly and constraint of the structure equation system are complete. Factoring and global solution now follow:

.
 .
 .

CALL FACT(ISGN,RNAME2, INAME2)

CALL SIMULO(ENERGY, RNAME2, INAME2)

At this point, vector (RNAME2, INAME2) contains the solution q for the complete structure. Part of q is, in fact, the boundary displacement solution q_B for the substructure. An interior solution may be obtained by continuing with:

CALL QBACK(LNUM,RNAME1, INAME1,RNAME2, INAME2)

.
 .
 .

After execution of QBACK, the substructure global solution ($q_I; q_B$) appears in vector (RNAME1, INAME1). The solution will also be

printed by subroutine QBACK.

2.3 Experiment with Conventional Mesh

The first attempt to model the panel and fastener hole was made with conventional quadrilaterals and a 5-node hybrid mesh-expander element. Experience with analysis of an attachment lug detail [1] indicated that at least 24 quadrilaterals should be placed around the hole to obtain accurate solutions. One then faces the following choices for the panel:

1. Mesh expansion in the near-field region to reduce the number of divisions below 24 on the near/far-field boundary (ABCD in Figs. 1 and 2).
2. Acceptance of 24 divisions on the near/far-field boundary and continuation of this detail into the far-field region.
3. Acceptance of 24 divisions on the boundary, with some mesh expansion immediately outside the near-field region.

The first choice was judged to be unlikely to give accurate computed displacements near the hole because of the severe shape distortions which the mesh-expander elements would have caused. The second choice was deemed to be unacceptable in that too many unwanted degrees of freedom would be placed in the far-field region, making either one-stage solution or substructuring computationally inefficient. The third choice was consequently selected as the best compromise between computational efficiency and solution accuracy near the hole.

Figure 4 illustrates a typical mesh for a panel with the fastener hole centered. Extra detail is kept in the far-field

region to the right of the hole in order to permit the hole to be offset continuously toward the right edge of the panel while avoiding extreme aspect ratios in the far-field elements. Tests with this model quickly showed that the asymmetric mesh grading led to highly asymmetric behavior of the computed solutions. Figure 5 illustrates a typical example. Computed horizontal displacement of the left and right edges of the panel are shown for the case of uniform vertical tension and the fastener hole centered. The inaccuracy of the model was judged to have been caused by the combination of asymmetric mesh grading, aspect ratio effects, and shape distortions in the near-field region.

The conclusion was that conventional finite-element models could be made to work for the panel structure only with very fine detail, and therefore at an unacceptably high computing cost. Lest it be thought that too much accuracy has been demanded from the analysis, we remind the reader that more than an engineering solution is required to achieve the ultimate goal. In considering the accuracy of the K_I and K_{III} solutions, it has been shown [1, 3] that shape distortions of the PCRK59 element alone can account for as much as 3 to 5 percent error. Furthermore, direct verification of accuracy by comparison with independent methods is possible only for K_I with cracks emanating horizontally from the fastener hole. Hence, a finite-element model capable of computing highly accurate displacements near the fastener hole is required to achieve engineering accuracy in the final answer. As a result, the assumed-stress hybrid method was investigated as a possible

alternate way of handling the transition from the circular hole geometry to the cartesian geometry of the near/far-field boundary.

2.4 Creation of a Hybrid Fastener Hole Element

The energy principles on which hybrid elements are based have been reviewed previously [1]. The energy principle Π_1 was chosen for the present investigation, giving the general form of an element stiffness matrix as:

$$\underset{\sim}{k} = \underset{\sim}{G}^T \underset{\sim}{H}^{-1} \underset{\sim}{G} \quad (9)$$

where

$$\underset{\sim}{G} = t \int_{\partial A} (\underset{\sim}{N} \underset{\sim}{P})^T \underset{\sim}{L} dA \quad (10)$$

$$\underset{\sim}{H} = t \int_A \underset{\sim}{P}^T \underset{\sim}{S} \underset{\sim}{P} dA \quad (11)$$

and where

A = Element area

∂A = Element boundary

$\underset{\sim}{L}$ = Matrix of assumed-displacement interpolation functions defined on ∂A .

$\underset{\sim}{N}$ = Matrix of direction cosines for the outward normal to ∂A .

$\underset{\sim}{P}$ = Matrix of assumed-stress shape functions.

$\underset{\sim}{S}$ = Matrix of elastic compliance constants.

t = Element thickness (assumed to be unity without loss of generality).

Two fundamental properties of hybrid elements make them useful in linear elastic plane stress and plane strain analyses:

1. Great flexibility in choice of element shape is permitted, since L need be defined only in terms of an arc length coordinate from node to node along ∂A , and since the shape functions P need only satisfy the continuum equilibrium equations.
2. Many shape functions P can be chosen to satisfy equilibrium and at the same time allow the element to closely mimic a particular local stress distribution.

Both properties have been used to advantage in the development of an element for the near-field region.

Practical application of the hybrid method requires that the analyst account for known local behavior in the formulation of his element. In the present case, one reasonably expects to see $\sin(2\theta)$ and $\cos(2\theta)$ components in the stress distribution near a fastener hole in a panel under tension, as well as terms independent of θ . Second, one should expect to see the near/far-field boundary deform in the manner shown schematically in Fig. 6. Simulation of this behavior requires that the element have at least three nodes (corners and mid-edge) along its near/far-field boundary edge. Third, the element must be kinematically stable, since it will be placed in the mesh so as to separate two conventionally modelled regions. Kinematic stability is assured if:

$$m \geq n - 3$$

(12)

for plane stress and plane strain elements, where

m = Total number of assumed-stress shape functions.

n = Total number of nodal degrees of freedom (displacements) in the element.

Finally, at least 20 nodes should be provided along the circular inner boundary, which may serve either as the fastener hole boundary or as an interface to which quadrilaterals and PCRK59 elements may be coupled.

The kinematic stability condition results in a quick determination that modelling of the entire near-field region with a single element would be impractical. For example, assumption of the minimum number of nodes (8 along the outer, 20 along the inner boundary) results in a 56-degree of freedom element which requires $m \geq 53$ assumed-stress shape functions. It is seen from Eqs. 9 and 11 that H is an $m \times m$ matrix which must be inverted to compute the element stiffness matrix. Furthermore, H is fully populated. The single-element approach was rejected in view of the difficulty in finding 53 shape functions and the computational inefficiency associated with inversion of a 53×53 matrix.

A 90-degree segment of the near-field region was the first geometry for which an element development was attempted. The element shape, shown in Fig. 7, encompasses one quarter of the circular boundary and one edge of the near/far-field boundary. With 20 divisions allowed around the hole, the element possesses 6 inner nodes in addition to its 3 near/far-field boundary nodes.

The element can be subjected to rotation transformations and repeated assembly to model the entire near-field region. The kinematic stability requirement now leads to $m \geq 18 - 3 = 15$ for the minimum number of stress shape functions. However, since rotation transformations are contemplated, insensitivity to orientation enters as another practical requirement. This leads to a choice of $m=16$, as explained in Subsection 2.5. Tests of the 90-degree element highlighted a problem of inaccuracy in the integration for the \underline{H} matrix (Eq. 11), which was judged to have been caused by the presence of terms in $\sin(4\theta)$ and $\cos(4\theta)$ in the stress shape functions \underline{P} .

The approach finally adopted divides the near-field region into eight 45-degree elements. Figure 8 illustrates the basic element, which possesses 4 nodes along its circular boundary. The element possesses 12 degrees of freedom, and therefore $m \geq 9$ is required. To preserve orientation insensitivity, $m=10$ is chosen, so that all $\sin(2\theta)$ and $\cos(2\theta)$ terms are included in \underline{P} . The entire near-field region is assembled by first computing \underline{k}_1 for the basic element and then subjecting \underline{k}_1 to a series of transformations for the seven other elements shown in Fig. 9.

2.5 Final Formulation of Hybrid Hole Element

The choice of assumed-displacement interpolation functions for the hybrid element is dictated by the need to maintain inter-element compatibility along its edges. Since the hole element must be capable of coupling to a quadrilateral or a PCRK59 segment between each pair of nodes, linear interpolation must be used.

In general,

$$\begin{Bmatrix} u(A) \\ v(A) \end{Bmatrix} = \underset{\sim}{L} \underset{\sim}{q} = \begin{bmatrix} 0 & 0 & \dots & 0 & 1-A/l & 0 & A/l & 0 & 0 & \dots & 0 \\ 0 & 0 & \dots & 0 & 0 & 1-A/l & 0 & A/l & 0 & \dots & 0 \end{bmatrix} \begin{Bmatrix} q_1 \\ q_2 \\ \vdots \\ q_{12} \end{Bmatrix} \quad (13)$$

where q_1, q_2, \dots, q_{12} are defined in Fig. 8, and where

$u(A)$ = Horizontal edge displacement.

$v(A)$ = Vertical edge displacement.

l = Length of edge between a pair of nodes.

The arc coordinate A in Eq. 13 is a relative measure of distance along any edge, with the positive sense taken as counterclockwise for integration along ∂A . Note also that the nonzero terms in $\underset{\sim}{L}$ appear in different positions for different edges. For example,

$$\underset{\sim}{L}_{34} = \begin{bmatrix} 0 & 0 & 0 & 0 & 1-A/l & 0 & A/l & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1-A/l & 0 & A/l & 0 & 0 & 0 & 0 \end{bmatrix} \quad (14)$$

for the edge between nodes 3 and 4, while

$$\underset{\sim}{L}_{61} = \begin{bmatrix} A/l & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1-A/l & 0 \\ 0 & A/l & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 1-A/l \end{bmatrix} \quad (15)$$

for the edge between nodes 6 and 1. In a similar manner,

$$\begin{aligned} A &= 0, l_{34} \text{ at nodes } 3,4 \\ A &= 0, l_{61} \text{ at nodes } 6,1 \end{aligned} \quad (16)$$

and so forth. These changes of definition do not cause difficulties in the boundary integration (Eq. 10) because the computation is carried out numerically, one edge at a time.

Assumed-stress shape functions are selected most conveniently by direct adaptation from classical elasticity solutions in polar coordinates. Let an Airy stress function $\Phi(r, \theta)$ be defined by:

$$\tau_{rr} = \frac{1}{r} \frac{\partial \Phi}{\partial r} + \frac{1}{r^2} \frac{\partial^2 \Phi}{\partial \theta^2} \quad \tau_{\theta\theta} = \frac{\partial^2 \Phi}{\partial r^2} \quad \tau_{r\theta} = \frac{1}{r^2} \frac{\partial \Phi}{\partial \theta} - \frac{1}{r} \frac{\partial^2 \Phi}{\partial r \partial \theta} \quad (17)$$

Then the equations of elasticity for homogeneous isotropic material reduce to:

$$\nabla^2 \nabla^2 \Phi(r, \theta) = 0 \quad (18)$$

where

$$\nabla^2 = \frac{\partial^2}{\partial r^2} + \frac{1}{r} \frac{\partial}{\partial r} + \frac{1}{r^2} \frac{\partial^2}{\partial \theta^2} \quad (19)$$

By assuming as an expansion for the stress function:

$$\Phi(r, \theta) = \phi^{(0)}(r) + \sum_{n=1}^{\infty} \phi^{(n)}(r) [\sin(n\theta) \text{ or } \cos(n\theta)] \quad (20)$$

it can be shown that Eq. 18 reduces to a set of equidimensional ordinary differential equations in $\phi^{(0)}(r)$, $\phi^{(1)}(r)$, ..., for

which the following solutions are obtained [7]*:

$$\phi^{(0)}(r) = a_0 + b_0 \ln r + c_0 r^2 + d_0 r^2 \ln r \quad (21)$$

$$\phi^{(n)}(r) = a_n r^n + b_n r^{-n} + c_n r^{2+n} + d_n r^{2-n}; \quad n \geq 2 \quad (22)$$

Stress distributions corresponding to Eqs. 21 and 22 are obtained by substituting back into Eqs. 17 [7]:

$$\begin{aligned} \tau_{rr}^{(0)} &= \frac{b_0}{r^2} + 2c_0 + d_0(1+2\ln r) \\ \tau_{\theta\theta}^{(0)} &= -\frac{b_0}{r^2} + 2c_0 + d_0(3+2\ln r) \\ \tau_{r\theta}^{(0)} &= 0 \end{aligned} \quad (23)$$

and

$$\begin{aligned} \tau_{rr}^{(n)} &= [a_n(n-n^2)r^{n-2} - b_n(n+n^2)r^{-n-2} + c_n(2+n-n^2)r^n + \\ &\quad + d_n(2-n-n^2)r^{-n}] [\sin(n\theta) \text{ or } \cos(n\theta)] \\ \tau_{\theta\theta}^{(n)} &= [a_n(n^2-n)r^{n-2} + b_n(n^2+n)r^{-n-2} + c_n(2+3n+n^2)r^n + \\ &\quad + d_n(2-3n+n^2)r^{-n}] [\sin(n\theta) \text{ or } \cos(n\theta)] \\ \tau_{r\theta}^{(n)} &= [a_n(n^2-n)r^{n-2} - b_n(n^2+n)r^{-n-2} + c_n(n^2+n)r^n + \\ &\quad - d_n(n^2-n)r^{-n}] [-\cos(n\theta) \text{ or } \sin(n\theta)] \end{aligned} \quad (24)$$

*The solution for $\phi^{(1)}(r)$ is of no interest in the present work. Terms in $\sin\theta$ and $\cos\theta$ correspond to problems in which a discontinuity has been introduced, e.g., re-welding of an annulus after a segment has been cut out.

The undetermined coefficients b_0, c_0, \dots, d_n in Eqs. 23 and 24 play the role of assumed-stress degrees of freedom, which are eliminated when the element stiffness matrix is formed [1]. The remaining $r\theta$ -dependent portions of the terms are placed in the shape function matrix \underline{P} .

Shape functions have been chosen from Eqs. 23 and 24 as follows. For the θ -independent terms, logarithmic behavior is not particularly desirable; only the b_0 and c_0 terms have been retained. This choice is admittedly arbitrary. To complete \underline{P} , enough θ -dependent terms are chosen from $2\theta, 4\theta, 8\theta, \dots$ behavior to satisfy kinematic stability and orientation insensitivity. The latter requirement demands, e.g., that all 2θ -dependent terms be retained if any are required for stability. It is apparent from Eqs. 24 that there are eight $n\theta$ -dependent terms for each n when $\sin(n\theta)$ and $\cos(n\theta)$ terms are included. Thus, selection of the 2θ -dependent terms provides a total of 10 shape functions, just enough to satisfy the requirements for the 45-degree element (The 90-degree element requires addition of the 4θ -dependent terms as well, making a total of 18 shape functions.). The shape function matrix, as finally adopted for the 45-degree element, is given by:

$$\underline{P}(r, \theta) = \begin{Bmatrix} -2\cos 2\theta & -6\cos 2\theta/r^4 & 0 & -4\cos 2\theta/r^2 & -2\sin 2\theta \\ 2\cos 2\theta & 6\cos 2\theta/r^4 & 12r^2\cos 2\theta & 0 & 2\sin 2\theta \\ 2\sin 2\theta & -6\sin 2\theta/r^4 & 6r^2\sin 2\theta & -2\sin 2\theta/r^2 & -2\cos 2\theta \\ -6\sin 2\theta/r^4 & 0 & -4\sin 2\theta/r^2 & 1/r^2 & 2 \\ 6\sin 2\theta/r^4 & 12r^2\sin 2\theta & 0 & -1/r^2 & 2 \\ 6\cos 2\theta/r^4 & -6r^2\cos 2\theta & 2\cos 2\theta/r^2 & 0 & 0 \end{Bmatrix} \quad (25)$$

The numerical integration required for computation of the \underline{H} matrix (Eq. 11) is accomplished most conveniently in polar coordinates. However, computation of \underline{G} (Eq. 10) actually requires the cartesian surface tractions which correspond to the assumed-displacement field:

$$\underline{T} = \{T_x \ T_y\} = \underline{N} \{\sigma_{xx} \ \sigma_{yy} \ \sigma_{xy}\} = \underline{N} \underline{P} \underline{\beta} \quad (26)$$

where $\underline{\beta}$ represents the vector of stress unknowns [1]. Hence, the following modification of Eq. 10 is required. Since

$$\underline{\tau} = \{\tau_{rr} \ \tau_{\theta\theta} \ \tau_{r\theta}\} = \underline{P} \underline{\beta} \quad (27)$$

in the present case, a Mohr circle transformation must be introduced:

$$\underline{\sigma} = \underline{M} \underline{\tau} = \underline{M} \underline{P} \underline{\beta} \quad (28)$$

and Eq. 10 is replaced by:

$$\underline{G} = t \int_{\partial A} (\underline{N} \underline{M} \underline{P})^T \underline{L} \, dA \quad (29)$$

where

$$\underline{M} = \begin{pmatrix} \cos^2 \theta & \sin^2 \theta & -\sin 2\theta \\ \sin^2 \theta & \cos^2 \theta & \sin 2\theta \\ \frac{1}{2} \sin 2\theta & -\frac{1}{2} \sin 2\theta & \cos 2\theta \end{pmatrix} \quad (30)$$

In Eq. 30, θ is the angular coordinate of the current integration station on ∂A . Thus, the price paid for convenience in choosing

$P(r, \theta)$ is the slight amount of additional computation required to form M at each integration station, as the element boundary ∂A is swept in Eq. 29.

2.6 Transformation-Assembly Procedure

A complete set of stiffnesses for the near-field region can be obtained from a single element computation by means of a transformation-assembly sequence. Element number 1 in Figs. 8 and 9 is always assumed to be oriented with edge 61 along the positive x-axis. Eqs. 11 and 29 are integrated with Gauss-Lagrange quadrature (GLQ) formulas [8] to compute k_1 , which is then assembled into a near-field substructure in accordance with the global node numbering scheme in Fig. 9.

It is apparent that the stiffnesses for elements 3, 5 and 7 can be obtained by rotation transformation of k_1 . For example, element 3 is oriented at +90 degrees from element 1. Hence, the transformation:

$$\{q_1, q_2, \dots, q_{12}\}_L = R \{q_1, q_2, \dots, q_{12}\}_G \quad (31)$$

relates the locally oriented displacements q_L to the globally oriented displacements q_G , where:

$$\tilde{R} = \begin{bmatrix} \cos\theta & \sin\theta & & & \\ -\sin\theta & \cos\theta & & & \\ & & \cos\theta & \sin\theta & \\ & & -\sin\theta & \cos\theta & \\ & & & & \ddots & \\ & & & & & \cos\theta & \sin\theta \\ & & & & & -\sin\theta & \cos\theta \end{bmatrix} \quad (32)$$

(Super-diagonal)

(12x12)

An example for one pair of nodal displacements is illustrated in Fig. 9. Since strain energy is independent of any particular coordinate system, a global stiffness matrix for element 3 may be computed as follows:

$$\text{Strain Energy} = \frac{1}{2} \underline{q}_L^T \underline{k}_1 \underline{q}_L = \frac{1}{2} \underline{q}_G^T \underline{k}_3 \underline{q}_G \quad (33)$$

Introducing Eq. 31 for \underline{q}_L leads to

$$\underline{q}_G^T \underline{R}^T \underline{k}_1 \underline{R} \underline{q}_G = \underline{q}_G^T \underline{k}_3 \underline{q}_G \quad (34)$$

for arbitrary values of \underline{q}_G . Therefore:

$$\underline{k}_3 = \underline{R}^T \underline{k}_1 \underline{R} \quad ; \quad \theta = \pi/2 \quad (35)$$

and in a similar manner,

$$\underline{k}_5, \underline{k}_7 = \underline{R}^T \underline{k}_1 \underline{R} \quad ; \quad \theta = \pi, 3\pi/2 \quad (36)$$

The sequence of rotation transformations given by Eqs. 35 and 36 assembly according to Fig. 9 is followed.

A stiffness matrix for element 2 can be obtained from \underline{k}_1 by the reflection transformation illustrated in Fig. 10. If element 2 is considered relative to a reflected xy axis system, then its stiffness is identical to \underline{k}_1 with the displacements oriented as indicated by the broken arrows. However, stiffness \underline{k}_2 is wanted again with respect to q_G . It is evident from Fig. 10 that for this case:

$$\underline{q}_1 = \underline{\Lambda} \underline{q}_G \quad (37)$$

where

$$\underline{\Lambda} = \begin{bmatrix} 1 & & & & \\ & -1 & & & \\ & & 1 & & \\ & & & -1 & \\ & & & & \text{(Diagonal)} \\ & & & & & 1 & -1 \end{bmatrix} = \underline{\Lambda}^T \quad (38)$$

(12 x 12)

Applying the strain energy argument again then gives immediately:

$$\underline{k}_2 = \underline{\Lambda} \underline{k}_1 \underline{\Lambda} \quad (39)$$

After \underline{k}_2 has been assembled, the procedure is completed with rotation transformations for:

$$\underline{k}_4, \underline{k}_5, \underline{k}_8 = \underline{R}^T \underline{k}_2 \underline{R} \quad ; \quad \theta = \pi/2, \pi, 3\pi/2 \quad (40)$$

where R is given by Eq. 32. The transformation-assembly procedure has been programmed internally, and the final result may be thought of as a single element with 16 outer boundary nodes, 24 inner boundary nodes, and 64 total degrees of freedom. Figure 11 illustrates the numbering convention for the assembly, which will be referred to as HOLEL in subsequent discussion.

2.7 Performance Tests

Several performance tests were conducted, in addition to routine checking of inversion accuracy in the H matrix, in order to assess the capabilities of HOLEL. Although its primary function is to serve as a link between a few inner rings of quadrilaterals and a coarse far-field mesh, there is some interest in determining the solution accuracy for structures in which the inner boundary of HOLEL is stress-free. Errors are to be expected for this situation, since the assumed surface tractions $N \ M \ P \ \beta$ generally do not vanish when $P(r, \theta)$ is computed for points lying on the element boundary ∂A . Errors of this type usually appear as overshoots in the computed stresses, given by [1]:

$$\underline{\tau}(r, \theta) = \underline{P}(r, \theta) \underline{H}^{-1} \underline{G} \underline{f}_L = \underline{B}_1(r, \theta) \underline{f}_L \quad (41)$$

where r, θ represent any point in the element domain A or on the boundary ∂A . Matrices $\underline{B}_1(r, \theta)$ were computed during the formation of \underline{k}_1 for several points along a ray at $\theta = 22.5$ degrees in element 1 (Fig. 9) for the purpose of the test.

Figs. 13, 14 and 15 illustrate the results of a test series in which the dimensions of HOLEL are (see Fig. 17):

Hole Diameter $D_o = 2$ inches

Edge Dimension $W = 8$ inches

The value $W/D_o = 4$ was chosen on the basis of a classical elasticity solution which indicates that far-field conditions are achieved at $r/R_o = 4$ for an open hole in an infinite plate [9]. The finite-element results computed from Eq. 41 are compared with the classical solution at $\theta = 22.5$ degrees [7].

The effect of increasing the number of GLQ integration stations is studied in Fig. 13, which plots $\tau_{\theta\theta}$. This stress component is the least sensitive to overshoot error because it does not enter into the stress-free condition on the inner circular portion of the boundary. The results show that 3 GLQ stations are too few, while acceptable accuracy is obtained with 4 or more stations. The programming of HOLEL was subsequently fixed at 5 GLQ stations based on these results. The meaning of "5 stations" is actually:

5 stations x 6 edges to compute G on $2A = 30$

5 x 5 stations to compute H in A = 25

Total GLQ integration stations = 55

This amount of computing is comparable to the amount required for the PCRK59 element.

Figures 14 and 15 illustrate the behavior of σ_{rr} and $\sigma_{\theta\theta}$, respectively. Finite-element results are shown only for 5 GLQ stations. These results demonstrate surprisingly that HOLEL is able to achieve the stress-free condition. However, some inaccuracy

can be observed at $r/R_0 \approx 1.7$. The important conclusion to be drawn from this test series is that HOLEL can be used without interior elements to model a multi-fastener-hole structure for which, e.g., local stress distributions or K_I and K_{II} solutions are sought only at one fastener hole, or at some other location.

The second test series studied the performance of HOLEL as a function of the parameter W/D_0 , using the test problem illustrated in Fig. 12. Results for $\tau_{\theta\theta}$ are plotted in Fig. 16. Some performance degradation is observed for $W/D_0 = 3$, as the near-field stress gradients begin to appear in the far-field elements, which are incapable of following this behavior with complete accuracy. Considerable degradation is also seen for $W/D_0 = 8$, a result which might be attributed to either the absence of far-field terms in $P(r, \theta)$, or errors in computing G and H caused by shape distortion, or both. One may conclude from these results that limitations must be placed on the edge distance and centerline spacing in HOLEL models of multi-fastener structures.

The objective of the third test series was to study the accuracy of HOLEL for the case of combined interference-fit or bearing loads and panel tension. Two tests were conducted, in which additional loads were applied at the fastener hole boundary of the model shown in Fig. 12. In Fig. 17, results for τ_{rr} are plotted for the case of 1 ksi panel tension combined with 1 ksi uniform pressure to represent an interference fit. A curve faired through the computed stresses extrapolates to -1.07 ksi at the fastener hole boundary, i.e., an error of about 7 percent. In

Fig. 18, results for τ_{rr} are plotted for 1 ksi panel tension combined with a bearing pressure distribution given by:

$$p(\theta) = \frac{2P}{\pi R_0} \sin \theta ; \quad (42)$$

$$P = 8,000 \text{ lb.} , \quad 0 \leq \theta \leq \pi$$

Eq. 42 gives

$$\tau_{rr} = -p(\theta) = -1.95 \text{ ksi} \quad (43)$$

at $\theta = 22.5$ degrees. In this case, a curve faired from 11 computed stresses extrapolates to the exact value at the fastener hole boundary. It is apparent from these results that HOLEL is accurate for both interference-fit and bearing loads. In the former case, the results could have been improved by computing τ_{rr} at 11 locations instead of the 5 which had been used. Far-field element stresses were also checked for this test series. In the case of interference-fit, the computed values for cartesian stress in the far-field elements were $\sigma_{yy} = 1 \text{ ksi}$ and $\sigma_{xx} = \sigma_{xy} \approx 0$ (to the roundoff level of the computer), with very little disturbance. In the case of bearing, it must be recognized that the 8,000-lb. bearing load adds to the 16,000-lb. panel tension load (1 ksi x 16-inch edge) for the far-field elements below the fastener hole. The computed stresses were in fact $\sigma_{yy} = 1 \text{ ksi}$ for the upper elements, $\sigma_{yy} = 1.5 \text{ ksi}$ for the lower elements, and $\sigma_{xx} = \sigma_{xy} \approx 0$, with load-transfer effects appearing in the far-field elements adjacent to HOLEL.

In the fourth test series, four rings of quadrilateral and PCRK59 elements were added to the structure model with the outer ring coupled to the circular boundary of HOLEL. Two test cases were analyzed. In the first case, the ring consisted entirely of quadrilaterals, and a cosine bearing pressure distribution (Eq. 42) was applied at the free boundary of the innermost ring. Figure 19 plots the results for τ_{rr} computed at the centroids of one "slice" of four quadrilaterals, along a ray at $\theta = 97.5$ degrees. The faired-curve extrapolation of these four data points agrees well with

$$\tau_{rr} = -p(97.5^\circ) = -\frac{2P}{\pi R_o} \sin(97.5^\circ) \approx -25.2 \text{ ksi} \quad (44)$$

In the second case, one or two groups of four quadrilaterals were replaced by PCRK59 elements to simulate a fastener hole with a crack at $\theta = 0$ or two equal-length cracks at $\theta = 0, \pi$. Several cases were analyzed covering the parameter ranges:

Hole diameter/HOLEL Edge: $0.02 < D_I / W < 0.05$

HOLEL edge/HOLEL diameter: $W/D_o = 4$

Crack length/hole radius: $0.25 < a/R_I < 1.0$

Table 1 compares the computed K_I values with independent classical solutions [10, 11]. The results demonstrate that the finite-element model is capable of providing reasonably accurate K_I solutions for quite extreme geometries (small fastener hole in a large panel).

The final test series consisted of a number of demonstration runs of the complete PANEL program, in which K_I and K_{II} solutions

were computed for another range of W/D_0 values. These results are presented in Section 5, following the documentation of the HOLEL procedure and the PANEL program.

Section 3

HOLEL PROCEDURE

Due to the complexity of the model, HOLEL has been programmed as several interrelated subroutines. Subroutine HOLEL provides overall control of the stiffness generation-assembly-transformation procedure, calling upon subroutines QHOLEL, GMTRX, HMTRX, LMTRX, MNMTRX, PMTRX and TRIG to execute specific computational tasks. In addition, HOLEL uses some of the FEABL-2 software [6] for the assembly process.

3.1 Structure Model and Input/Output Conventions

A square near-field region with an inner circular boundary of radius R_0 is modelled. The numbering conventions have been indicated in Fig. 11. Subroutine HOLEL is invoked by:

CALL HOLEL(COORD,THK,S,RI,RSS,ISS,B)

where the arguments are dimensioned and defined as follows:

- COORD(12) - Vector of cartesian coordinates of the element corners in order $X_1, Y_1, Z_1, X_2, \dots, Z_4$. (In the present version, the element is assumed to lie in the XY plane, and the Z coordinates need not be given any values.)
- THK - Scalar value of element thickness.
- S(3,3) - Array of elastic compliance constants for homogeneous isotropic material, i.e.:

$$\underline{S} = \frac{1}{E} \begin{bmatrix} 1 & -\nu & 0 \\ -\nu & 1 & 0 \\ 0 & 0 & 2(1+\nu) \end{bmatrix} \quad (45)$$

RI - Scalar value of inner boundary radius R_0 .
 RSS(2097), - Floating-point and fixed-point names of a
 ISS(2097) FEABL-2 DATA vector. These arguments must
 be equivalenced, as well as dimensioned in
 the user's MAIN program.

B(6,3,13) - A collection of \underline{B} matrices for stress analysis
 (see Subsection 3.2).

HOLEL returns the assembled subregion stiffnesses as a variable-bandwidth-stored matrix in the DATA vector RSS,ISS. The results may be read into another storage area with the algorithm given in Appendix A, or may be assembled directly into another DATA vector with FEABL-2 subroutine ASMSUB (see Subsection 2.2).

Procedure HOLEL requires no input data cards.

3.2 Required Subprograms and Other Features

Procedure HOLEL requires the following additional software for execution:

1. ASRL FEABL-2 subroutines ASMLTV, ORK and SETUP.
2. IBM Scientific Subroutine Package subroutines MFSD and SINV.

No external disk or tape files are required.

3.3 Model Generation and Program Flow

Figure 20 summarizes the program flow in Procedure HOLEL. At entry, whatever control parameters are present in the FEABL-2 labelled COMMON areas SIZE, BEGIN and END are saved in temporary storage. Subroutine SETUP is called to establish address control for the RSS,ISS vector for a structure model with 64 total degrees of freedom, consisting of the eight 12-degree-of-freedom elements in Fig. 9. Element interconnections are then generated internally, in accordance with the numbering conventions in Fig. 8 and Fig. 11, and subroutine ORK is called to compute the band margin and complete address control for the assembled stiffness matrix.

Subroutine QHOLEL is now called to compute k_1 , the stiffness matrix for element 1. This subroutine calls in turn subroutines HMTRX and GMTRX to compute H^{-1} and G , forms the stiffnesses $G^T H^{-1} G$. Subroutines HMTRX and GMTRX loop over the GLQ integration stations in the element domain A and on ∂A respectively, calling upon subroutines TRIG, PMTRX, LMTRX and MNMTRX to compute $P(r, \theta)$, $L(A)$ and $M^T N^T$ at each station, and accumulating the product sums (including GLQ weighting factors) for $P^T SP$ and $(NMP)^T L$. Subroutine HMTRX calls SINV (which calls MFSD) to invert H .

After k_1 has been computed, subroutine QHOLEL forms a $6 \times 3 \times 13$ matrix for stress analysis, consisting of six matrices $B(r, \theta)$ as given by Eq. 41, and stored in accordance with the following conventions:

$$B(I, J, K) = \left[\underline{B}_i(r_i, \theta_i) \mid \begin{matrix} \theta_i \\ r_i \\ 0 \end{matrix} \right] \quad (46)$$

where subscript I identifies the specific \underline{B} matrix, and where

$$\begin{aligned}\theta_i &= \pi/8 \quad ; \quad i=1,2,\dots,6 \\ \tau_1 &= R_0 \\ \tau_i &= \tau_{i-1} + 0.2(\text{distance from } R_0 \text{ to outer} \\ &\quad \text{boundary along } \theta_i) ; \quad i=2,3,\dots,6\end{aligned}\tag{47}$$

Thus, the \underline{B} matrices refer to six locations which divide the ray $\theta = \pi/8$ into five equal segments, with \underline{B}_1 located on the inner circular boundary and \underline{B}_6 located on the near/far-field boundary. Polar stresses for these locations can be computed from:

$$\{\tau_{rr} \quad \tau_{\theta\theta} \quad \tau_{r\theta}\}_i = \text{STRESS}(J)_I = \sum_{K=1}^{12} B(I,J,K)^* Q(K) \tag{48}$$

where

$$Q(K) = \underline{z}_L \tag{49}$$

An extra column ($K=13$) is appended to each \underline{B} matrix for storage of the polar coordinates of its location.

At this point, subroutine HOLEL regains control and executes the transformation-assembly process outlined in Subsection 2.6. In the program, \underline{k}_2 is formed first, and the assembly sequence is programmed as:

Assembly of k_1 and k_2

Rotation through $\pi/2$ to k_3 and k_4

Assembly of k_3 and k_4

Rotation through π to k_5 and k_6

.
.
.

Assembly of k_7 and k_8

Finally, the HOLEL address control parameters are transferred to the FEABL-2 labelled COMMON areas SIZESS and BEGSS, and the user's control parameters are returned to areas SIZE, BEGIN and END. Control is now returned to the user's calling program, with HOLEL ready for assembly to the user's structure model by means of FEABL-2 subroutine ASMSUB.

3.4 Procedure Status

All of the HOLEL subroutines, together with copies of IBM subroutines MFSD and SINV, are maintained as a unit in the form of individually sequenced, 029-punched FORTRAN-IV source decks. A listing of Procedure HOLEL appears in Appendix B.

Section 4

PANEL PROGRAM

The PANEL program is an executive program which controls the assembly of several substructures into a model of a skin tension panel with a single open fastener hole. The PANEL program uses procedure HOLEL and several additional special-purpose procedures to generate the required substructures. Details of these additional procedures are discussed in Subsections 4.2 and 4.3.

4.1 Structure Model and Input Conventions

Figure 21 illustrates the structure which the PANEL program models: a rectangular panel loaded by uniform tension on its horizontal edges. The panel contains a single fastener hole centered vertically. The fastener hole may be either centered or offset horizontally. The vertical edges of the panel may be stiffened symmetrically with integral stiffeners, if desired. One or two cracks may be placed to emanate radially from the fastener hole. If two cracks are specified, they will be located 180 degrees apart, and they may be of equal or unequal length. The angular position, θ , to the first crack may be varied in 15-degree increments from 0 to 345 degrees if there is only one crack, or from 0 to 165 degrees if there are two cracks. A second version of the PANEL program is available for a structure in which only the left edge may be stiffened. The second version

is otherwise similar to the first version, and will not be discussed separately.

Four input data cards are required for the PANEL program. The input data describe the specific panel geometry and define several parameters which control the type of solution executed and the amount of information printed. Figure 22 illustrates the correct format for the input cards:

1. Panel parameters

WIDTH = Total width of the panel, W_p .

LENGTH = Total length of the panel.

THK = Panel thickness.

STFFCT = Stiffener factor.

PRESS = Tension loading, σ_{yy} .

RI = Radius of the fastener hole.

IOFFST = Offset indicator.

2. Crack parameters

A(1) = Length of first crack.

A(2) = Length of second crack.

IPOS(1) = Crack initial position number

IPOS(2) = Crack final position number.

3. Material properties

E = Young's modulus.

ν = Poisson's ratio.

4. Print control parameters

KT1 = Control for optional FEABL-2 output.

KT2 = Control for optional FARFLD output.

KT3 = Control for optional RING output.

The stiffener factor is defined in terms of the panel dimensions (Fig. 21):

$$STFFCT = w_r t_r / w_p t_p \quad (50)$$

The value $STFFCT = 0$ corresponds to an unstiffened panel. Negative values may be used to analyze panels with edges thinner than the primary structure. The offset indicator is used to control the type of solution desired. The following options are available:

$IOFFST = 0$ - Centered fastener hole only.

$IOFFST = 1$ - Fastener hole moves right.

$IOFFST = -1$ - Fastener hole moves left.

For the latter two options, solutions are executed automatically beginning with the hole on center and ending with the hole as close to the edge of the panel as permitted by the finite-element model. If $STFFCT \neq 0$, motion of the hole is further restricted to avoid overlapping with a stiffener.

A value of 0 should be assigned to crack length $A(2)$ if a structure with only one crack is to be analyzed. Crack sizes up to 1.27 (RI) are permitted. The crack position numbers control the angular location of the first crack as follows:

$$\begin{aligned} \theta_{initial} &= (IPOS(1) - 1) \Delta\theta \\ \theta_{final} &= (IPOS(2) - 1) \Delta\theta \\ \Delta\theta &= \pi/12 \text{ rad.} = 15 \text{ deg.} \end{aligned} \quad (51)$$

Permissible limits for the position numbers are:

$$1 \leq \text{IPOS}(1) \leq n_{\text{max}} \quad \text{IPOS}(1) \leq \text{IPOS}(2) \leq n_{\text{max}} \quad (52)$$

where

$$\begin{aligned} n_{\text{max}} &= 24 \quad \text{if } A(2) = 0 \\ n_{\text{max}} &= 12 \quad \text{if } A(2) \neq 0 \end{aligned} \quad (53)$$

Solutions are executed automatically beginning with the first crack in position 1 and ending with the first crack in position 2. A single solution is executed if $\text{IPOS}(2) = \text{IPOS}(1)$. A case matrix is executed if $\text{ICFFST} \neq 0$ and $\text{IPOS}(2) > \text{IPOS}(1)$.

Most of the routine information printed by the PANEL program is of no interest when production runs are executed. This information may be deleted from the output by assigning each of the control parameters KT1 , KT2 , KT3 values equal to the FORTRAN unit number for the line printer at the user's computing facility.* Any other value permits full output by the associated software. Full output is recommended for initial testing of the program at a new facility. Output from FARFLD and RING should be allowed whenever a new range of panel dimensions is tried.

4.2 Required Subprograms and Other Features

The PANEL program requires the following additional software for execution:

*The same value used in a print instruction, e.g., WRITE (6,1000) A,B,C. The FORTRAN unit number in this case is 6.

1. ASRL FEABL-2 subroutines ASMLTV, ASMSUB, BCON, FACT, ORK, QBACK, SETUP, SIMULQ, STACON and XTRACT.
2. IBM Scientific Subroutine Package subroutines MFSD and SINV.
3. ASRL procedures FARFLD, HOLEL and RING, with included subroutines.
4. ASRL elements PCRK59 and QUAD4.

No external disk or tape files are required. The program must be able to communicate with the user's facility card reader and line printer, by means of two instructions near the beginning of the program:

KR = Card reader FORTRAN unit number.

KW = Line printer FORTRAN unit number.

The program is supplied with IBM-standard values KR=5 and KW=6.

4.3 Model Generation and Program Flow

The panel structure model is created via several levels of substructuring to minimize repeated processing of unwanted degrees of freedom. Figure 23 illustrates the general hierarchy of the finite-element model. Procedures FARFLD and RING generate the two major components which are finally assembled to form the complete structure.

Procedure FARFLD creates the far-field region, consisting of upper and lower rectangular substructures generated by procedure LUG,* and a center-zone substructure generated by

*Different from the attachment lug procedure reported in Ref. 1.

procedure CZONE. Procedure LUG assembles a regular mesh of QUAD4 elements and eliminates all except the upper-edge and lower-edge nodes. Procedure CZONE assembles HOLEL together with right and left portions modelled with QUAD4 elements, and eliminates all but the upper- and lower-edge nodes and the inner circular boundary nodes. Procedure FARFLD assembles the LUG and CZONE components and executes additional Gauss elimination to produce one of two major substructures:

1. "Panel-and-Hole", in which only the nodes along the top and bottom edges of the panel and the nodes along the inner circular boundary remain as boundary nodes in the statically condensed structure. Displacement restraints are applied at the bottom edge and nodal forces are applied at the top edge to represent uniform tension.
2. "The Cheshire Cat": a panel-and-hole with top and bottom edges eliminated by static condensation (nothing remains but the smile).

Only the Cheshire Cat option is used by the PANEL program. Numbering conventions for the FARFLD components are illustrated in Figs. 24, 25 and 26.

Procedure RING creates an inner cracked ring for assembly inside the Cheshire Cat. The ring consists of two 150-degree arcs and two 30-degree segments which contain QUAD4 and PCRK59 elements. The arcs are assembled by procedure ARC4, which also

removes all interior nodes by static condensation. Procedure RING assigns the angular locations of the components to obtain the required crack positions, and determines where each PCRK59 element is to be placed in the 30-degree segments according to the crack sizes A(1), A(2). Finally, procedure RING assembles and statically condenses the cracked ring to produce one of two major substructures:

1. Ring with all but inner and outer circular boundary nodes eliminated.
2. Ring with all but outer circular boundary nodes eliminated.

Retention of the inner boundary nodes is useful for cases in which bearing or interference-fit loads are to be applied at the fastener hole. The outer boundary nodes must be retained for coupling with the Cheshire Cat. The PANEL program uses only the second option. Numbering conventions for the RING components are illustrated in Figs. 27 through 30. Procedure RING always generates a ring with an outer/inner diameter ratio of 2.52 to maintain shape conformity for the QUAD4 elements.

Figure 31 illustrates the executive flow in the PANEL program. After the problem input data have been read and printed and some parameters have been calculated for control of the various procedures, two major loops appear. The outer loop begins with the creation of a Cheshire Cat, a step which must be repeated each time the fastener hole is offset to a new position. This is followed by address control and interconnection generation for

the final structure, which will be assembled from the Cheshire Cat (super-element 1) and the cracked ring (super-element 2). The interconnection algorithm is actually completed inside the inner loop to allow the band-margin computations (Subroutine ORK) to erase the K and q data from the previous pass. The remainder of the inner loop consists of assembly of the Cheshire Cat, generation and assembly of the cracked ring, global solution (subroutines FACT and SIMULQ), a back-substitution to obtain q_L for the cracked ring substructure and finally, extraction of the PCRK59 displacements from q_L and computation of K_I and K_{II} . The ends of the inner and outer loops are governed by incrementation of the crack angle and hole offset, respectively, and logical checks to determine whether the prescribed ranges of these parameters have been swept.

4.4 Output Conventions and Error Messages

Figure 32 illustrates a sample output from the PANEL program (version 2, left side stiffened). The output from Version 1 is similar. The heading identifies the program version and repeats the user's input data. Below the material properties data appears a table of K_I and K_{II} solutions for one or two cracks, together with their angular positions. The K_I and K_{II} values are NASA/ASTM standard stress intensities in units of $\text{psi } \sqrt{\text{in.}}$, assuming that the input data was specified in corresponding engineering units of psi for loading and Young's modulus and inches for dimensions. The sample output is an example of the production information obtained by exercising the three options for deletion of debugging output.

Besides the FEABL-2 software error messages [6], the following diagnostics may result from the PANEL programs. In procedure RING, the length specified for each crack is checked to insure that the crack tip does not extend beyond the average radius of the outermost ring of quadrilaterals, i.e.:

$$R_I + A(J) \leq \frac{1}{2} (R_4 + 2.52 R_I) ; \quad J=1,2 \quad (54)$$

where R_4 is the inner radius of the outermost ring. If Eq. 54 is violated, a message is printed and program execution is terminated. This condition is somewhat conservative, since it restricts the crack tip to a position mid-way in the PCRK59 element. Eq. 54 may be replaced by:

$$R_I + A(J) \leq 0.3 R_4 + 0.7 \times 2.52 R_I \quad (55)$$

to permit the crack tip to approach somewhat closer to the outer boundary. In procedure CZONE, a check is made to insure that the fastener hole offset is within allowable limits. This is done by monitoring the node number of the fictitious center reference node (see Fig. 25). Excessive offset causes an error message and program termination.

4.5 Program Status

Both versions of the PANEL program have been exercised successfully for all analysis options, and are maintained as

sequenced, 029-punched FORTRAN-IV source decks. Either version requires 260 to 300 KBYTES ($65,000_{10}$ to $75,000_{10}$ words, or $176,750_8$ to $222,370_8$ words) of core memory and approximately 0.8 to 1.0 CPU minute, depending upon the ranges of the hole offset and crack angle parameters. Storage and time statistics are based on runs made on an IBM S-370/168 machine, using the IBM FORTRAN-G1 and FORTRAN-H compilers. Version 1 (symmetrical edge stiffeners) is listed in Appendix C. Version 2 (left edge stiffener only) is listed in Appendix D.

Section 5

DEMONSTRATION EXAMPLES

A number of example analyses have been run to verify the PANEL program code and simultaneously to explore the accuracy of the analysis for a range of panel and crack dimensions wider than considered in the HOLEL performance tests. The solutions given here focus mainly on panels with centered fastener holes having cracks at $\theta=0,\pi$ because there exist no independent solutions with which to compare other configurations.

Figure 33 illustrates the coarsest model possible to generate from the PANEL program: lugs consisting of two elements each and a center-zone composed only of HOLEL. Also, the dimensions chosen are such that the fastener hole is no longer small compared to the panel, and since the outer/inner diameter ratio of the cracked ring is 2.52, the HOLEL shape parameter in this case is:

$$W/D_o = 4/2.52 \approx 1.59$$

a point well outside the range studied in the HOLEL performance tests. The butterfly plots for K_I and K_{II} shown in Fig. 33 illustrate an error effect caused by proximity of the displacement boundary conditions to the region of interest. In the present case, the restraints are only two elements away from the "action" (a lug QUAD4 and the HOLEL) when either crack lies below the

horizontal. The error is most pronounced for K_{II} at $\theta = 225, 315$ degrees. The error would be more pronounced as $\theta \rightarrow 270$ degrees, except that the solution tends rapidly to zero in this region. A similar but less-pronounced effect can be observed in the plot for K_I . Comparison of K_I for $\theta = 0, 180$ degrees with an independent solution [10, 11] provides a more welcome result. Indeed, it is quite surprising that this very crude finite-element model is able to faithfully reproduce the classical solution. The general conclusion to be drawn from this test is that only the upper half of the butterfly plot may be trusted when running models with very few elements in the FARFLD substructure.

Figures 34 and 35 illustrate two runs in which the panel and crack dimensions have been varied to explore the effect of W/D_0 . Also, the aspect ratios of the elements in the LUG substructure were changed to allow more elements between the restrained bottom edge and the center-zone. The latter modification has eliminated the restraint error effect. Table 2 summarizes the comparisons of results from Figs. 33 through 35 with the independent solution, showing that reasonable accuracy has been achieved over:

$$1.59 \leq W/D_0 \leq 6.35$$

The greater error for the middle case remains an unexplained anomaly.

Figure 36 illustrates the K_I and K_{II} solutions obtained for a panel of the same dimensions, hole radius, etc. shown in Fig. 35, but with the hole offset to the maximum amount permitted in

the model, a distance of 1.5 inches. Increases of the order of 5 to 10 percent are observed for K_I , with the greatest increase at crack tips which are located nearest to the edge of the panel.

Figures 37 through 39 present results for some additional studies of the basic panel shown in Fig. 35 to illustrate the stiffening options. A stiffener factor of 0.5 was used for these analyses, i.e., the added cross section area of each stiffener was 50 percent of the panel cross section area. Figure 37 plots K_I and K_{II} solutions for a symmetrically stiffened panel. Results are shown for the fastener hole centered and offset by 1 inch. There is very little difference between the two solutions. Solutions for a panel with left-edge stiffener and a centered fastener hole are plotted in Fig. 38. The results for K_I and K_{II} appear to be symmetrical. In fact, very little difference can be observed between the unstiffened, symmetrically stiffened and asymmetrically stiffened panels (compare Figs. 35, 37 and 38). Some difference is noted when the fastener hole is offset 1.5 inches to the right, away from the stiffened edge. Figure 39 presents the results for this case, which are almost identical to the results for an unstiffened panel (compare with Fig. 36). The tentative conclusion from these results is that edge stiffeners and moderate offsets do not appreciably affect K_I and K_{II} for cracks at the fastener hole, at least for uniform tension loading and when the hole and crack are not extremely close

to an edge or stiffener.

Figure 40 summarizes the dimensional information for a panel model used to study a broad range of a/R_I ratios with W/D_0 fixed at 1.59. Table 3 summarizes the results for a series of cases with equal-length cracks at $\theta=0, \pi$. These may be compared directly with handbook data because the dimensions of the model correspond precisely with a published curve. The results are seen to be quite reasonable over the entire range:

$$0.05 \leq a/R_I \leq 1.25$$

Indeed some of the error indicated in the table may be attributed to inaccuracy in reading the handbook chart. A number of other cases for a single crack at $\theta=0$ and for two unequal-length cracks are compared with the foregoing results in Table 4. Complete butterfly plots of the K_I and K_{II} data computed from these runs will appear in a later report.

Section 6

CONCLUSIONS

This report has traced the development of a parametric finite-element analysis program for computation of Mode I and Mode II stress intensity factors in stiffened and unstiffened panels with cracks emanating from centered and offset fastener holes. Early attempts to model the structure entirely with conventional elements, except for the crack-tip element and a few mesh-expander elements, proved to be unsuccessful. Test runs of these types of finite-element models quickly demonstrated that asymmetric grading of the mesh was forced by the need to accommodate an offset hole and at the same time to keep the size of the model within economic bounds. The asymmetric mesh was found to give extremely poor results for computed displacements in regions of low stress gradient, and was therefore abandoned.

The assumed-stress hybrid method was called upon once again, this time to provide a special element which could handle the transition from circular geometry near the fastener hole to the cartesian geometry natural to the rest of the panel. As finally developed, the hybrid element occupied a 45-degree sector between its inner (circular) and outer boundary. Airy stress functions from classical elasticity solutions near a hole in an infinite plate were used to provide the assumed stress field. The new

element allowed a much cleaner, more economical finite-element mesh to be designed, while at the same time providing accurate computations near the fastener hole. Performance tests have shown that the element is capable of occupying a stress-free fastener hole boundary directly, and of accepting interference-fit and cosine bearing loading on the fastener hole surface. Additional tests demonstrated that conventional quadrilateral elements could be coupled in rings inside the inner boundary of the new hybrid element, with no loss of accuracy. Some tests were also conducted with hybrid crack-containing elements replacing some of the quadrilaterals. These tests demonstrated that stress intensity factors could be computed to within a few percent of the values given by well-established independent solutions based on classical methods. In the final phase of the project, a parametric program was developed and verified for analysis of tension panels in the various configurations mentioned above. During the verification tests, the hybrid fastener hole-ring combination was subjected to a variety of dimensional and shape parameters to further extend the range of measured performance. The results of these tests have demonstrated that the panel program is capable of computing stress intensity factors to within 5 percent or better for fastener hole and crack sizes found in current airframes.

The many example results presented in this report are still a rather limited data base, when compared with the number of stress

intensity factor solutions needed for a comprehensive designer's handbook. Some additional data, which were generated in the final verification tests but not included in this report, will appear in a later report in this series. However, further verification tests are still required to broaden the range of applicability of the panel program. One concern which has not yet been answered is how close a row of fastener holes may be spaced, or how near may a fastener hole approach the edge of a panel, before the finite-element model experiences unacceptable degradation of accuracy. The data generated thus far seem to indicate that there will be a limit in this respect, and that the limit may be severe. Addition of mid-edge nodes to the edges which span between the hybrid fastener hole element's inner and outer boundaries may permit closer spacing, but will also require additional terms in the assumed stress field.

Another continuing concern is associated with the verification process itself. The combination of crack-containing and fastener hole elements gives the capability to compute K_I and K_{II} for so many varied configurations that the numerical analyst finds himself sailing in uncharted waters. At the present time, parametric codes like the panel program can only be calibrated against classical solutions for K_I at one or two data points, while they may compute as many as 100 data points each for K_I and K_{II} .

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TABLE 1
COMPARISON OF CLASSICAL AND FINITE-ELEMENT
FRACTURE MECHANICS SOLUTIONS

Number of Cracks	Crack Size, a (in.)	Hole Radius, R _I (in.)	a/R _I	K _I (psi/√in.)		% Error
				Exact	FEM	
2	0.04	0.08	0.5	650	630	3.0
1	0.04	0.10	0.4	660	675	2.4
2	0.08	0.10	0.8	792	771	2.7
2	0.05	0.20	0.25	910	910	---
2	0.20	0.20	1.0	1150	1170	1.9

TABLE 2
COMPARISON OF CLASSICAL AND FINITE-ELEMENT
RESULTS FROM THREE DEMONSTRATION EXAMPLES
(Fastener hole with 2 cracks)

Case	Fig.	Crack Size (in.)	Hole Rad. (in.)	a/R _I	HOLEL W/D ₀	K _I (psi/√in.)		% Error
						Exact	FEM	
1	11	0.1	0.5	0.6	1.59	1900	1900	1.0
2	14	0.125	0.25	0.5	1.17	1091	1134	4.1
3	15	0.125	0.125	1.0	6.15	886	860	0.7

TABLE 3
PERFORMANCE OF PANEL PROGRAM AS A
FUNCTION OF a/R_I RATIO

(Structure model in Fig. 40; cracks at $\theta=0, \pi$.)

a(in.) and a/R_I	K_I (psi $\sqrt{\text{in.}}$)		% Error
	Exact*	FEM	
0.05	1144	1160	1.4
0.1	1673	1613	3.6
0.2	2037	2051	0.7
0.4	2474	2445	1.2
0.6	2735	2715	0.7
0.8	2973	2897	2.6
1.0	3257	3099	4.9
1.2	3521	3401	3.4
1.25	3588	3500	2.5

*Ref. 11, p. 19.4, curve marked $h/b=2$, $R/b=0.25$.

TABLE 4
ADDITIONAL TEST OF a/R_I RATIO

(Structure model in Fig. 40; one crack or two unequal cracks.)

a(in.) and a/R_I		K_I (psi $\sqrt{\text{in.}}$) at $\theta=0$		% Difference
$\theta=0$	$\theta=\pi$	FEM	Exact (from Table 3)	
0.05	---	1155	1144	1.0
0.1	---	1600	1673	4.4
0.2	---	2000	2037	1.8
0.4	---	2302	2474	6.9
0.6	---	2466	2735	9.8
0.8	---	2522	2973	15.2
1.0	---	2602	3257	20.1
1.2	---	2758	3521	21.7
1.25	---	2813	3588	21.6
0.05	1.25	1447	1144	26.5
1.25	0.05	2824	3588	21.3

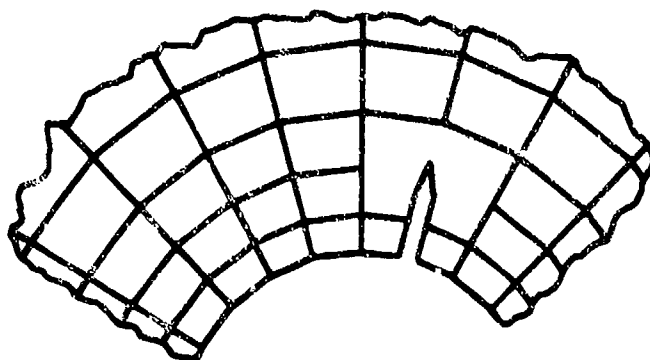
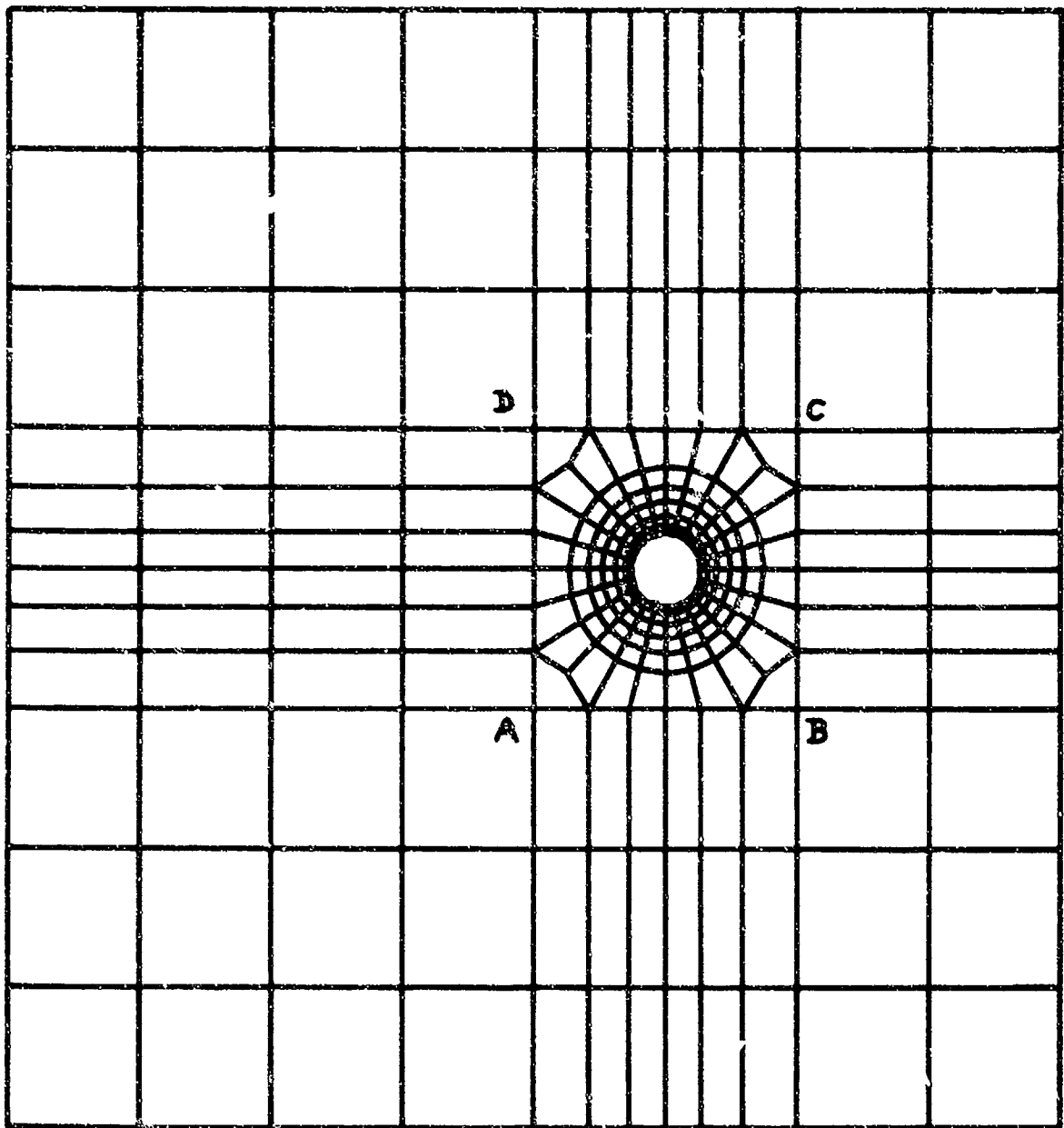


FIG. 1 CONVENTIONAL FINITE-ELEMENT MODEL OF PANEL WITH OFFSET FASTENER HOLE

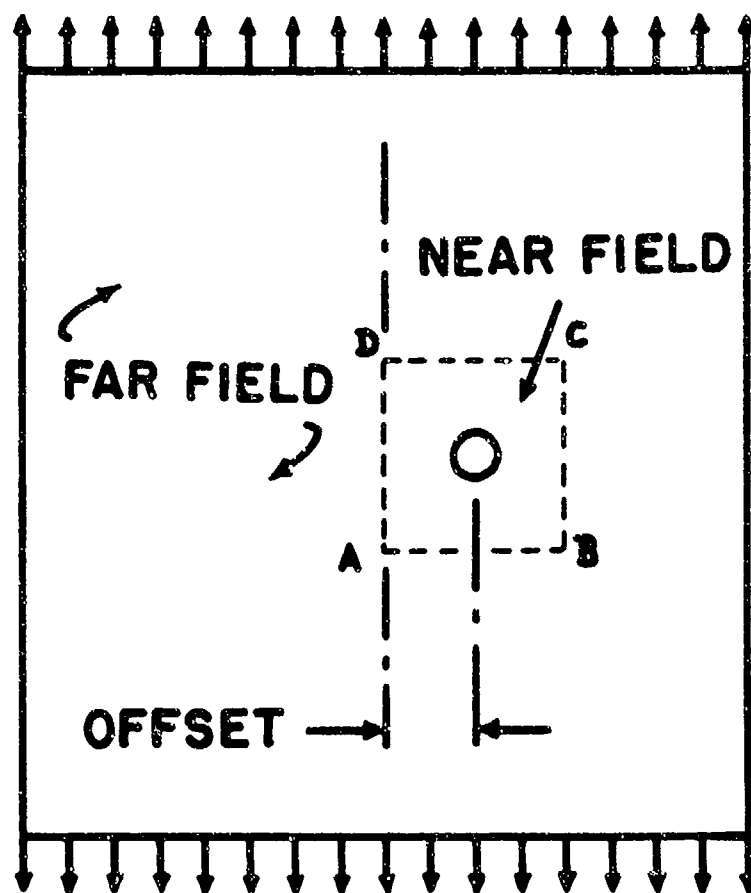


FIG. 2 SUBDIVISION OF PANEL INTO NEAR-FIELD AND FAR-FIELD REGIONS

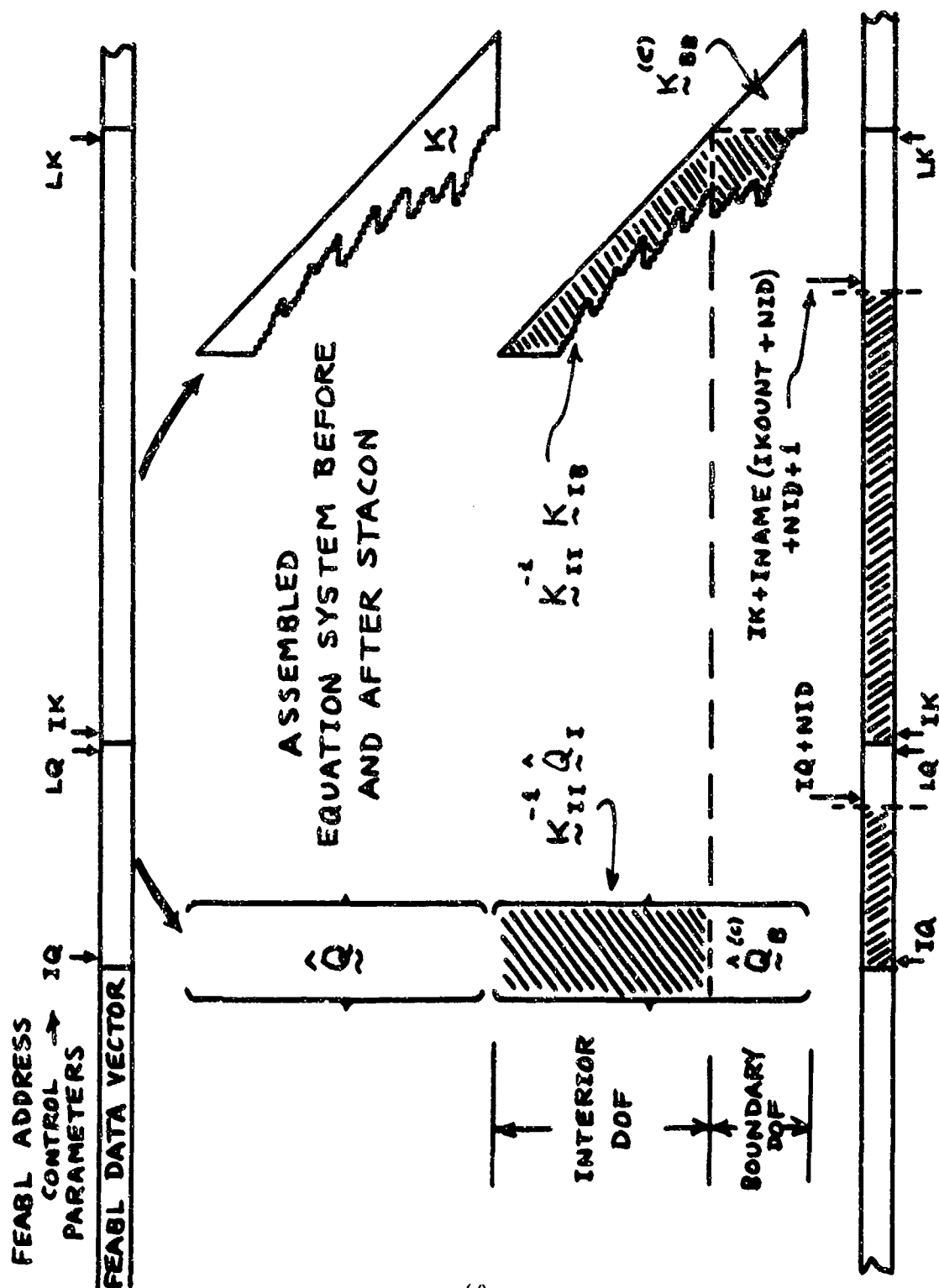
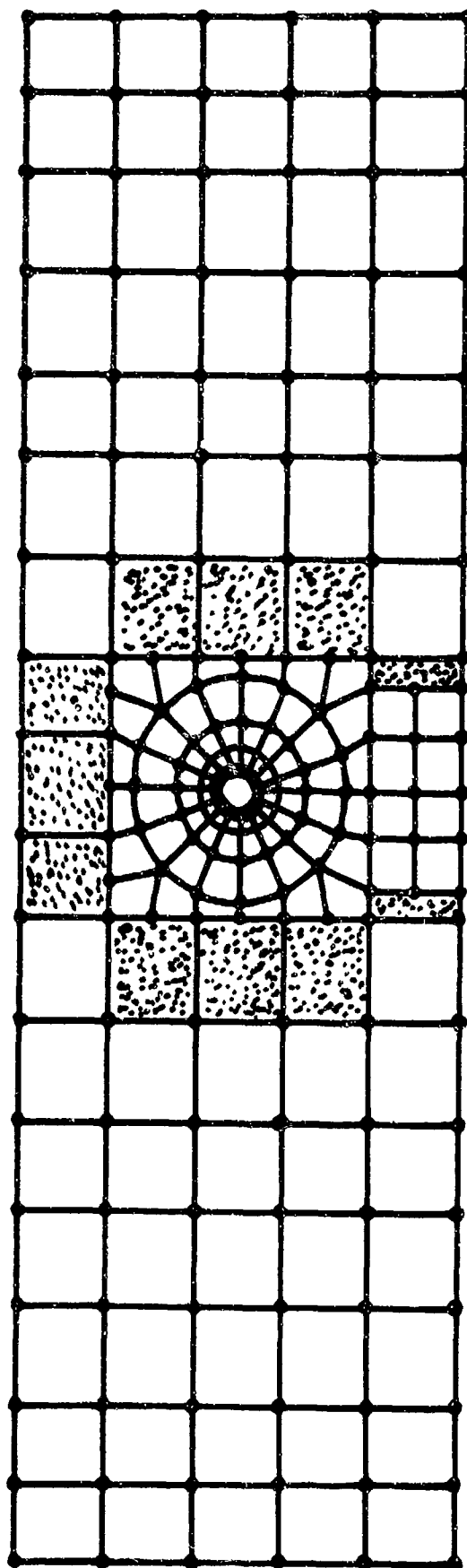



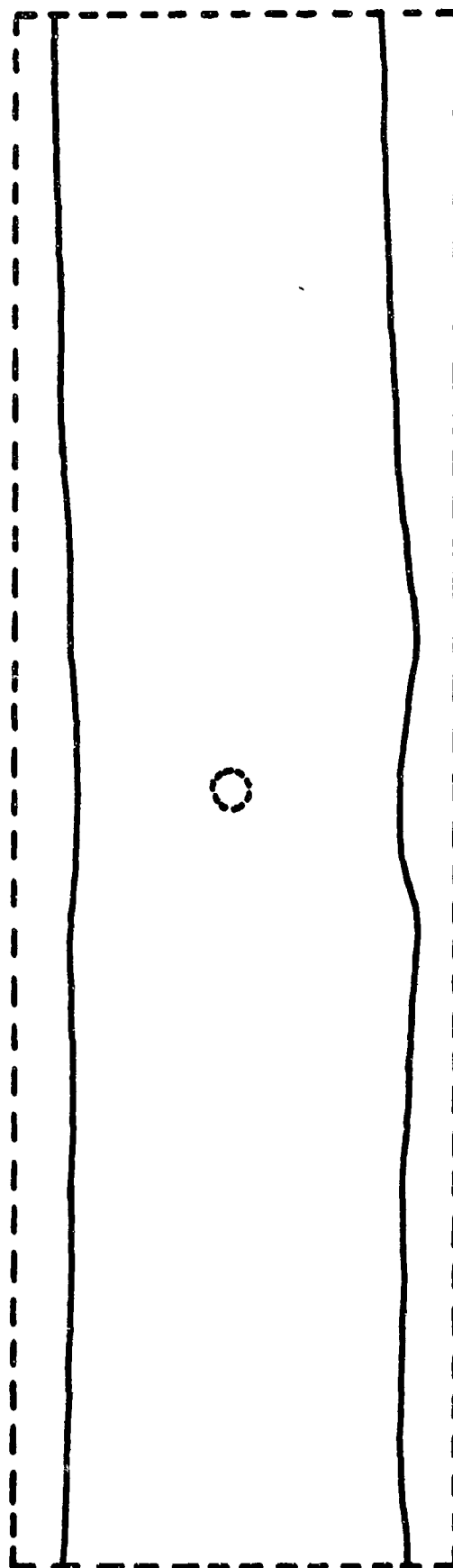
FIG. 3 ACTION OF FEABL-2 SUBROUTINE STACON ON ASSEMBLED SUBSTRUCTURE STIFFNESS MATRIX




HPOLY5
ELEMENT

376
DEGREES OF
FREEDOM

FIG. 4 PLOT OF CONVENTIONAL
 FINITE-ELEMENT PANEL-
 AND-HOLE MODEL TESTED
 FOR NUMERICAL ACCURACY

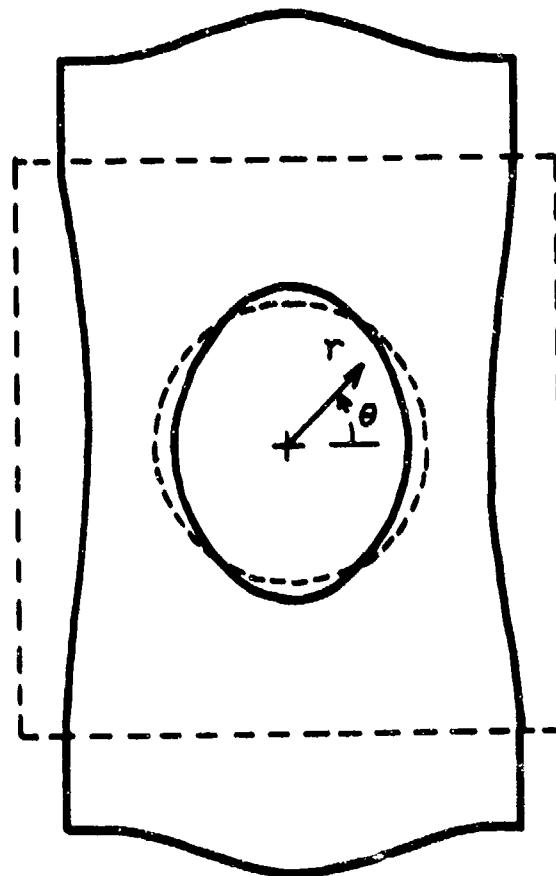


UNDEFORMED

VERTICAL EDGES

AFTER
DEFORMATION

FIG. 5 ILLUSTRATION OF
INACCURATE DIS-
PLACEMENT SOLUTION
CAUSED BY ASYMMETRIC
MESH GRADING



----- UNDEFORMED
———— DEFORMED

FIG. 6 EXPECTED DEFORMATION BEHAVIOR OF NEAR-FIELD REGION

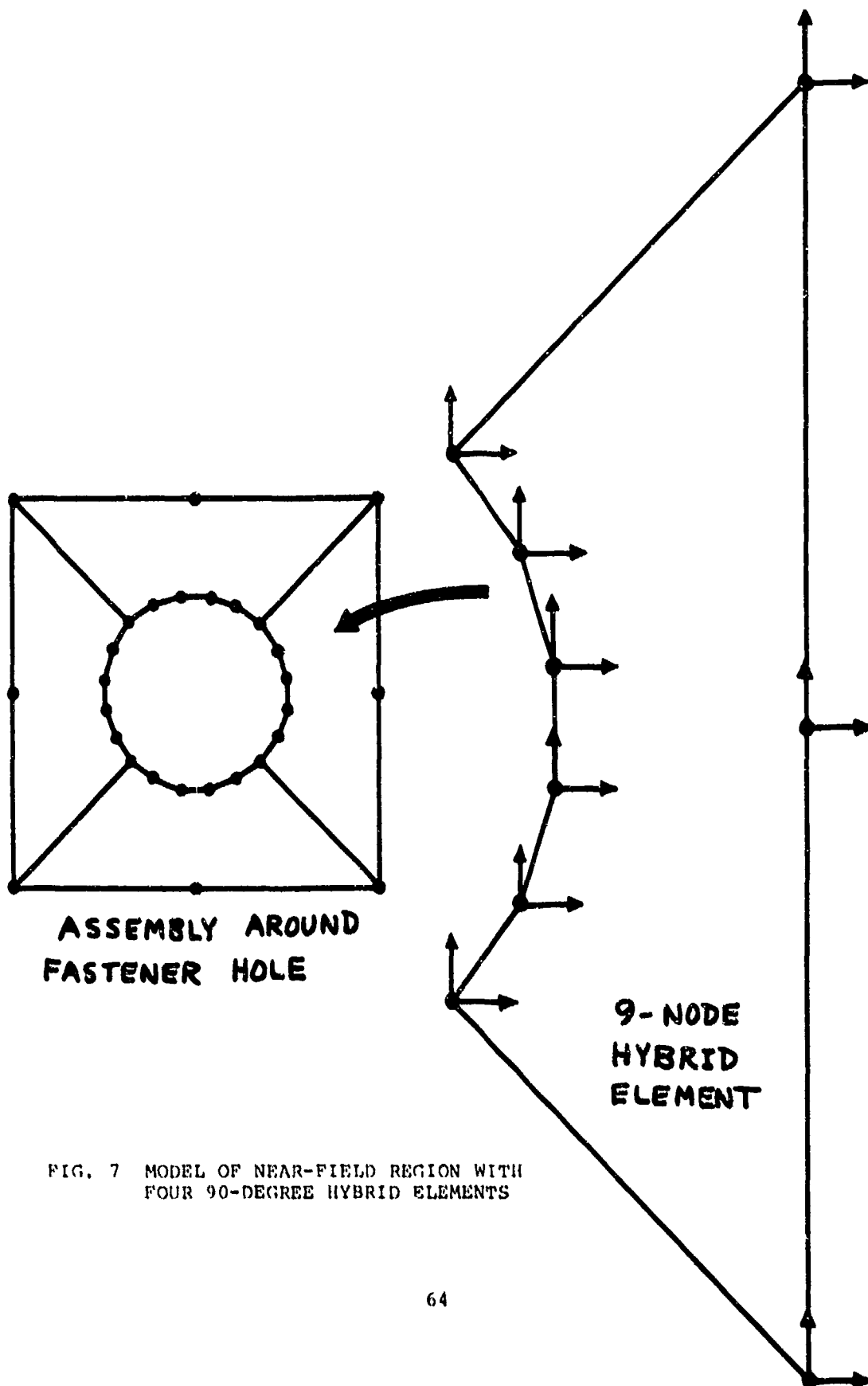


FIG. 7 MODEL OF NEAR-FIELD REGION WITH
FOUR 90-DEGREE HYBRID ELEMENTS

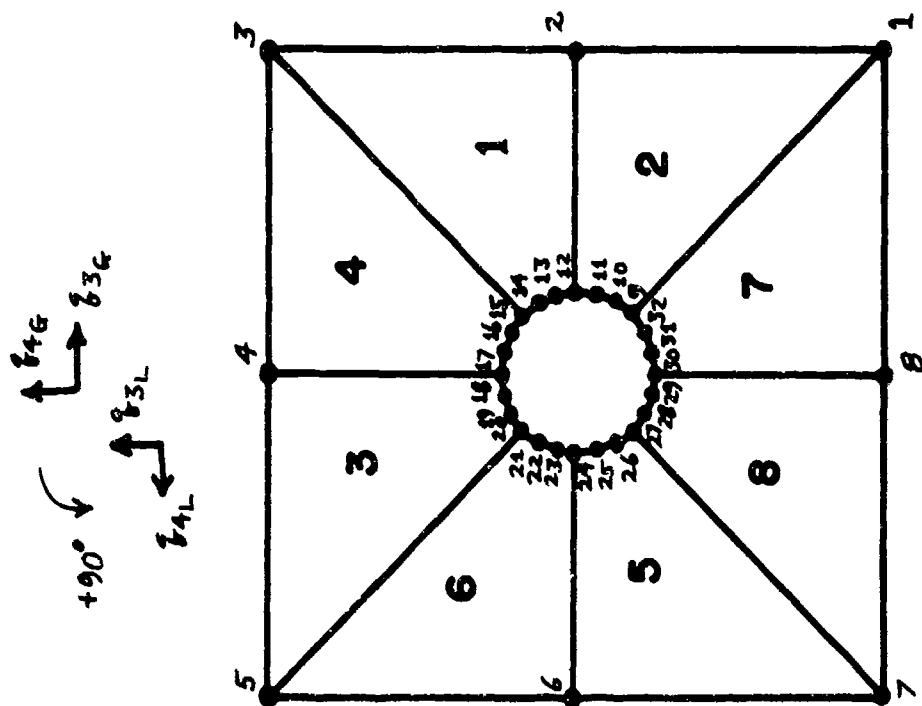


FIG. 9 MODEL OF NEAR-FIELD REGION WITH EIGHT 45-DEGREE HYBRID ELEMENTS

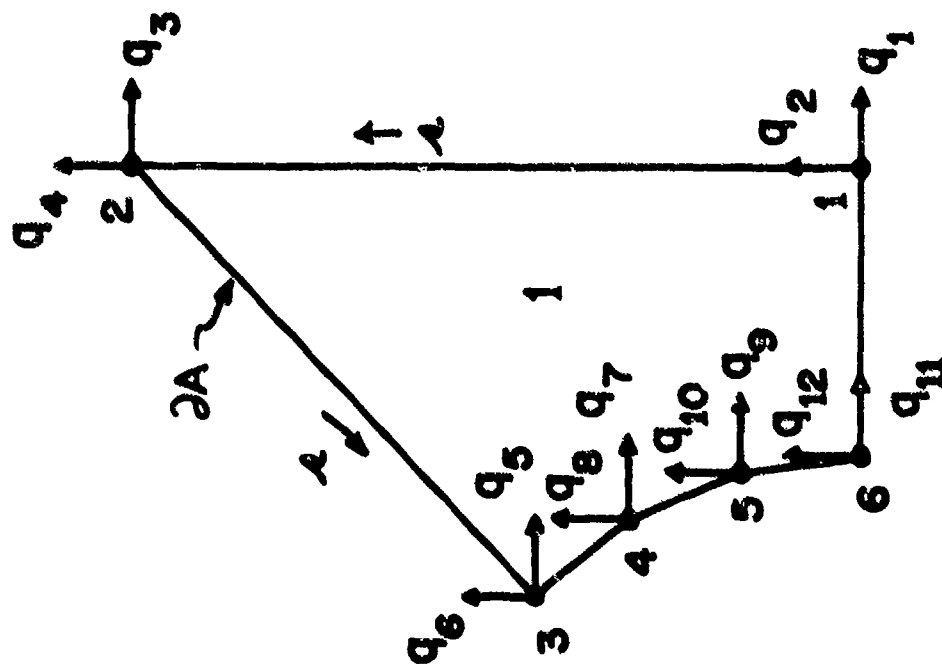


FIG. 8 LOCAL NUMBERING CONVENTION FOR 45-DEGREE HYBRID ELEMENT

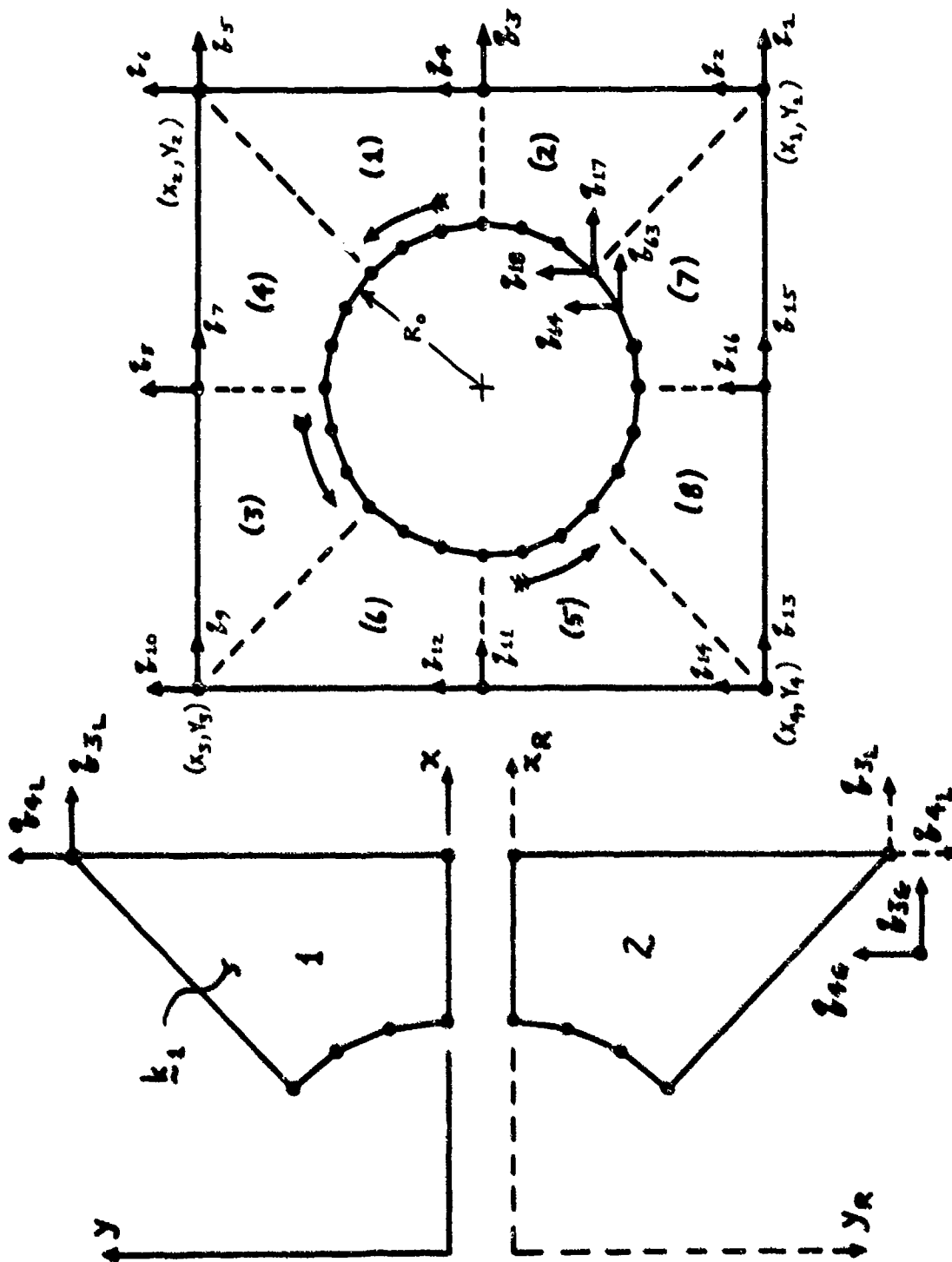


FIG. 10 EXAMPLE OF REFLECTION TRANSFORMATION

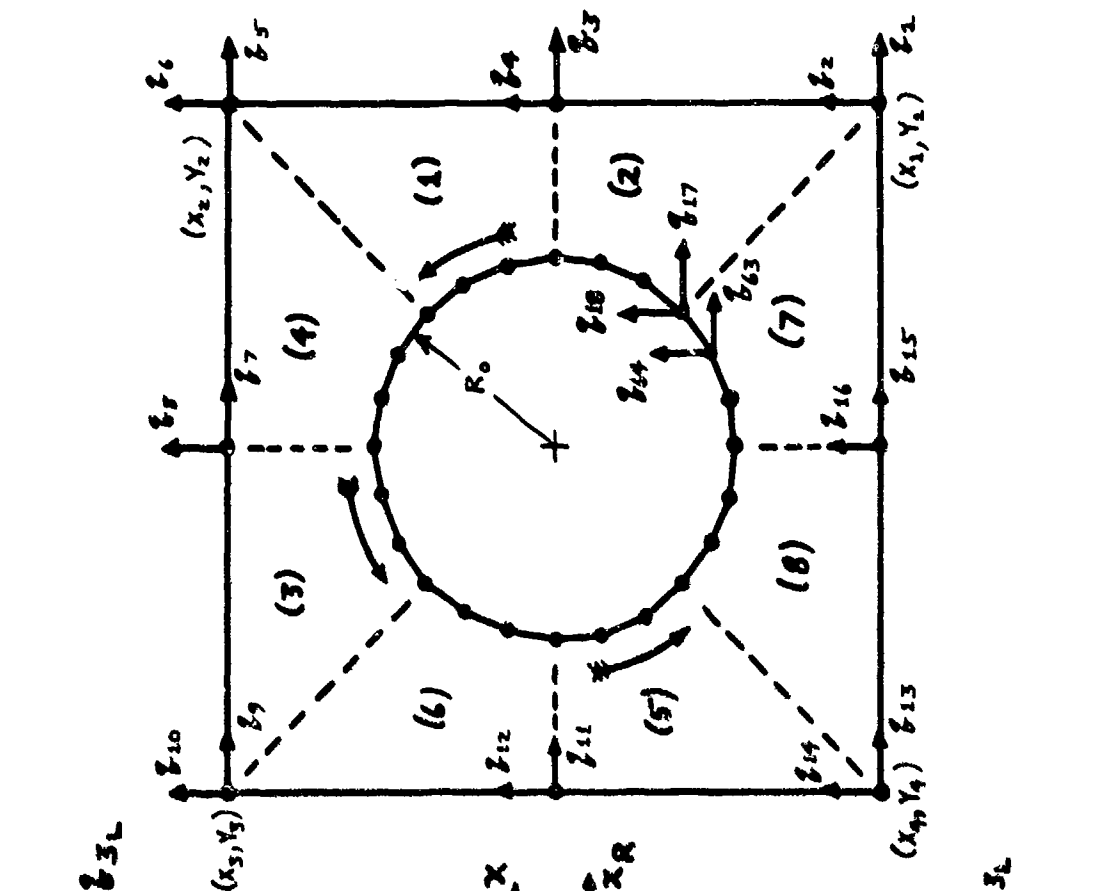


FIG. 11 LOCAL NUMBERING CONVENTION FOR COMPLETED HYBRID FASTENER HOLE ELEMENT (HOLE)

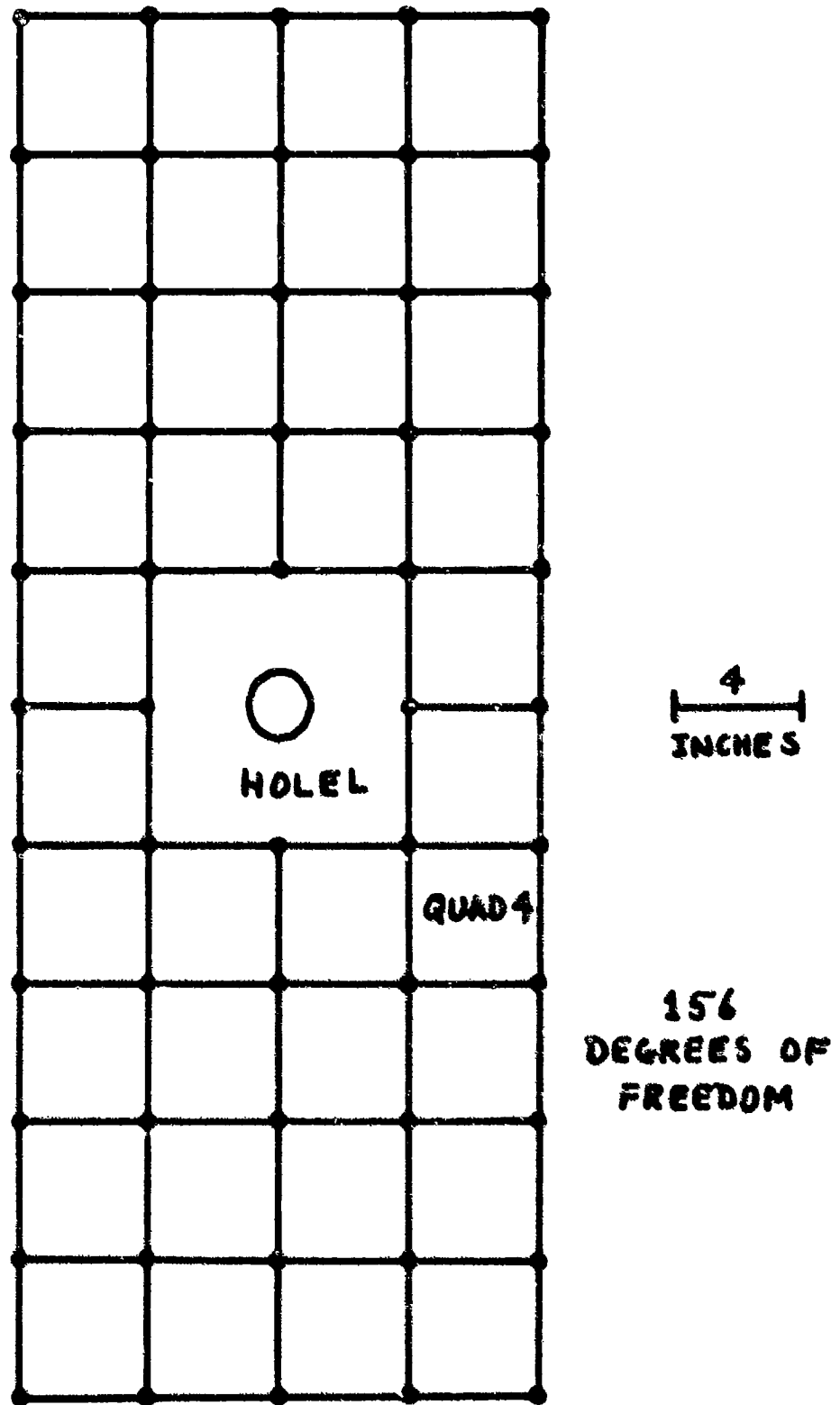


FIG. 12 FINITE-ELEMENT MODEL FOR PERFORMANCE TESTING OF HOLE

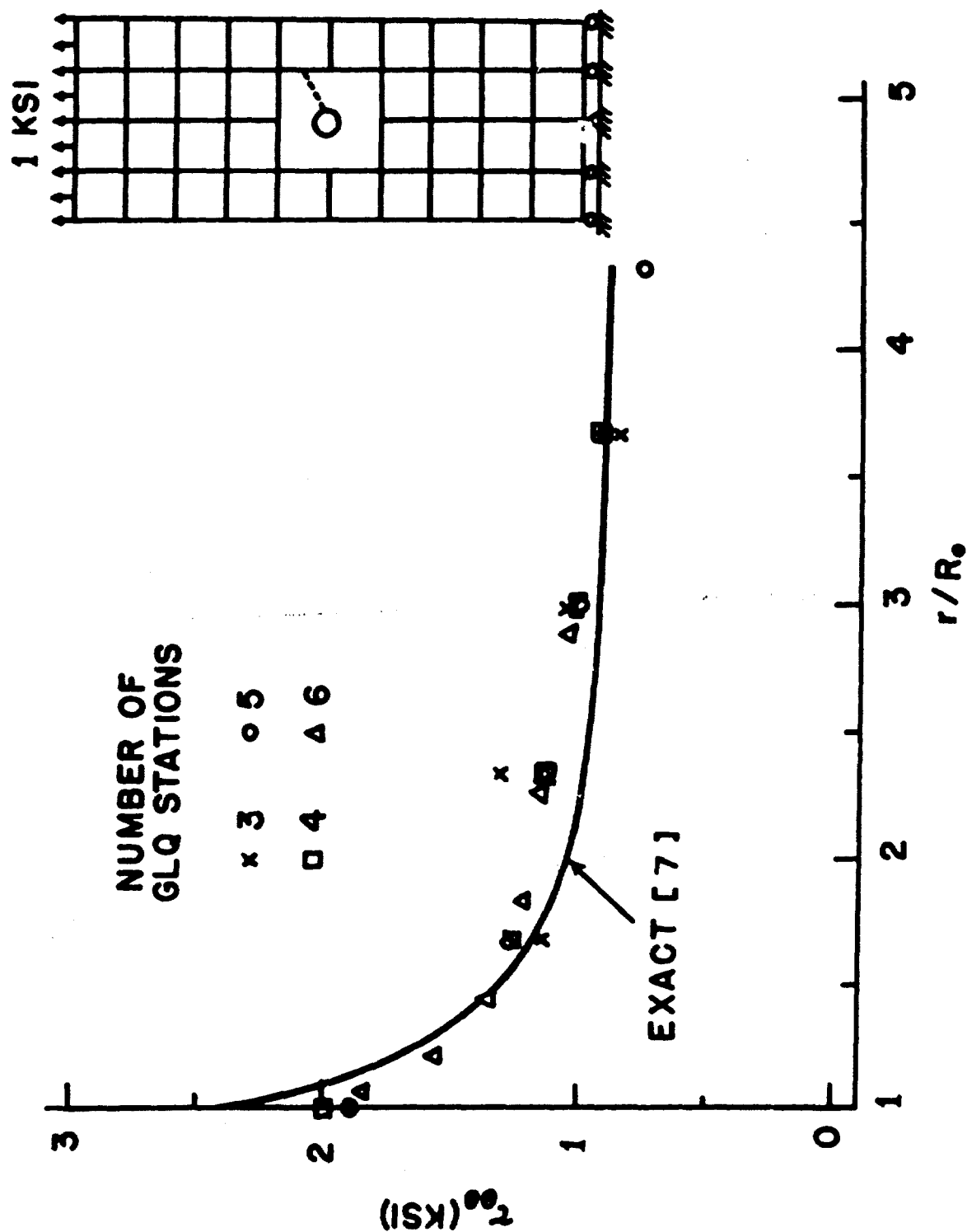


FIG. 13 SENSITIVITY OF HOLE TO NUMBER OF GLQ INTEGRATION STATIONS

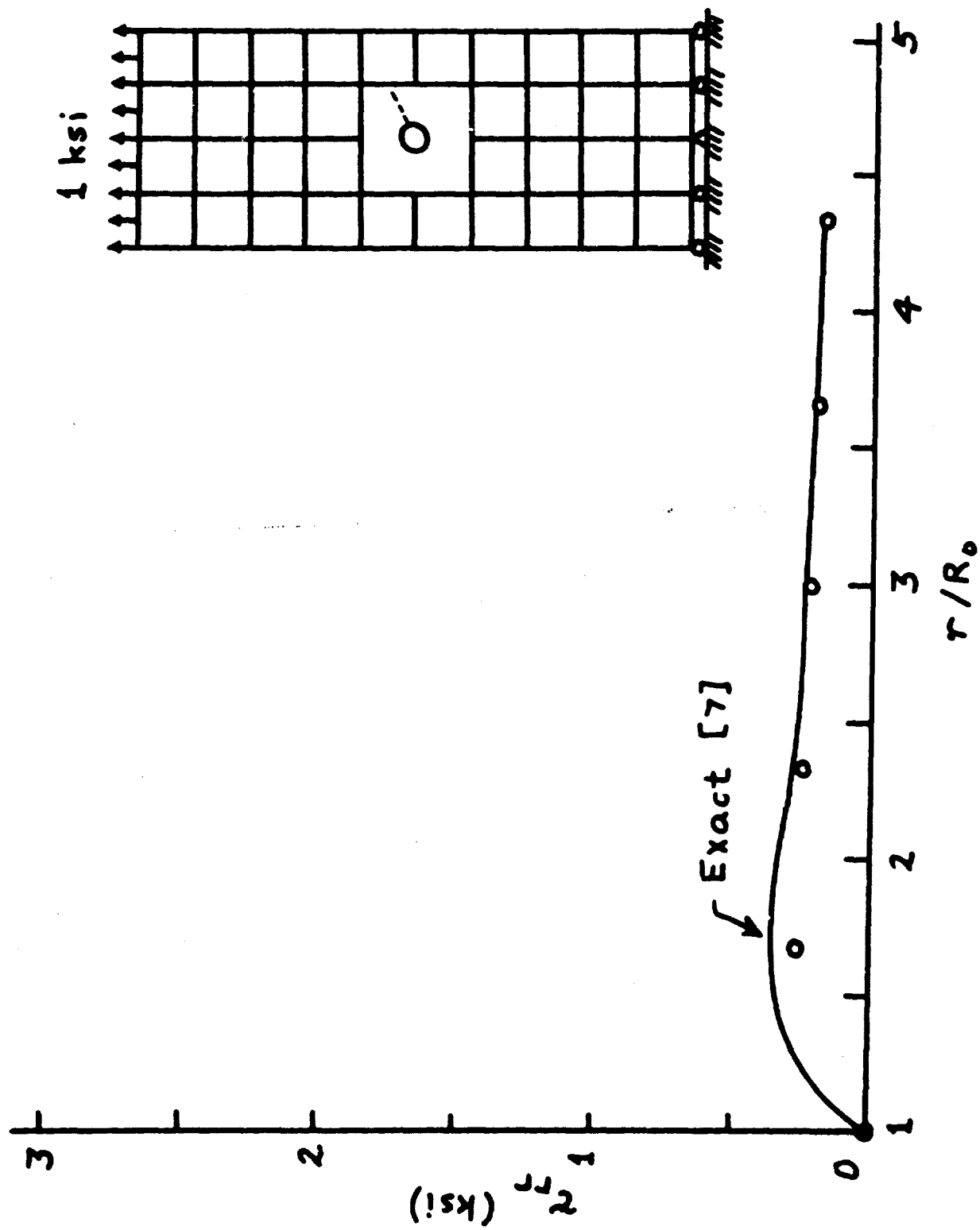


FIG. 14 BEHAVIOR OF τ_r NEAR STRESS-FREE EDGE

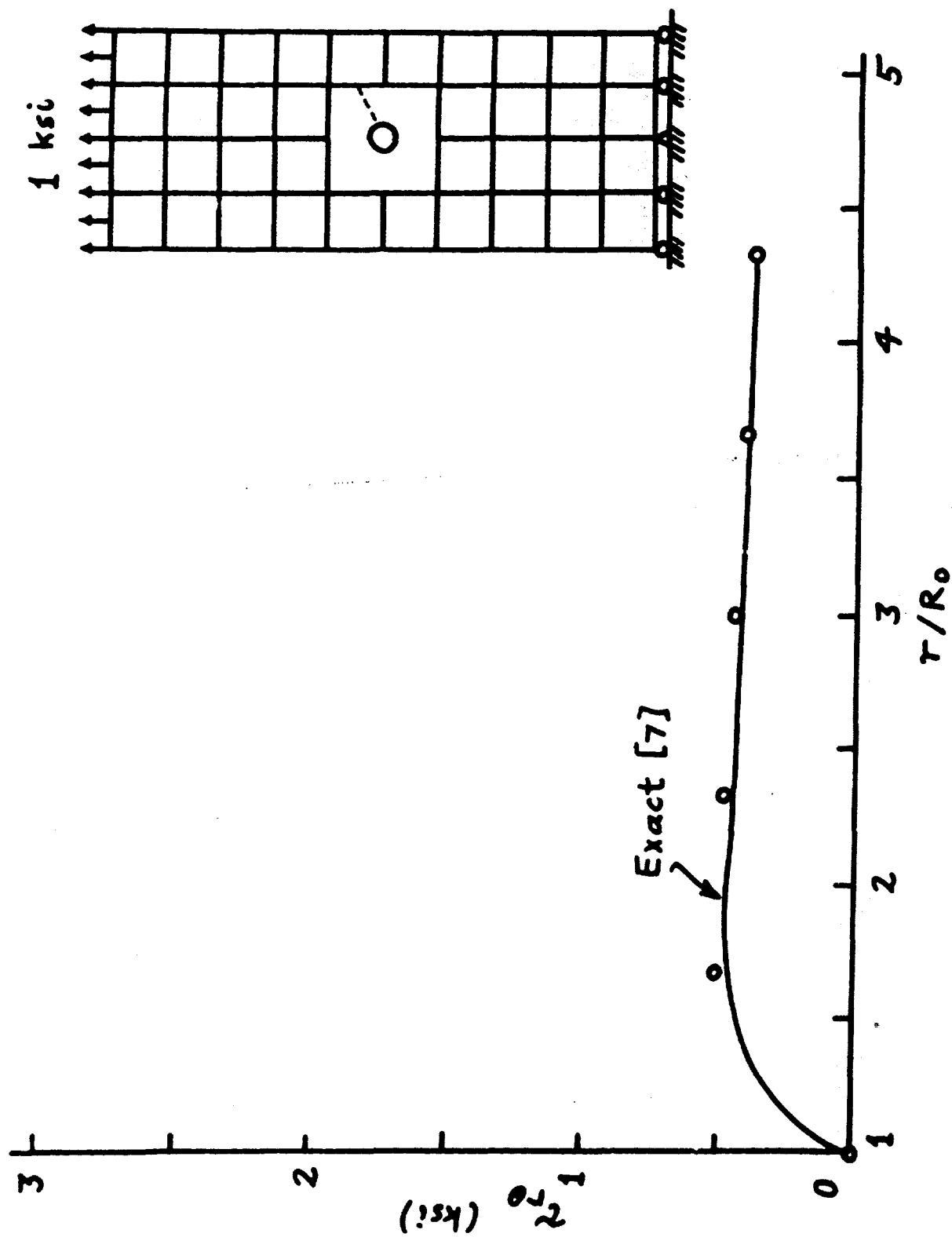


FIG. 15 BEHAVIOR OF $\tau_{r\theta}$ NEAR STRESS-FREE EDGE

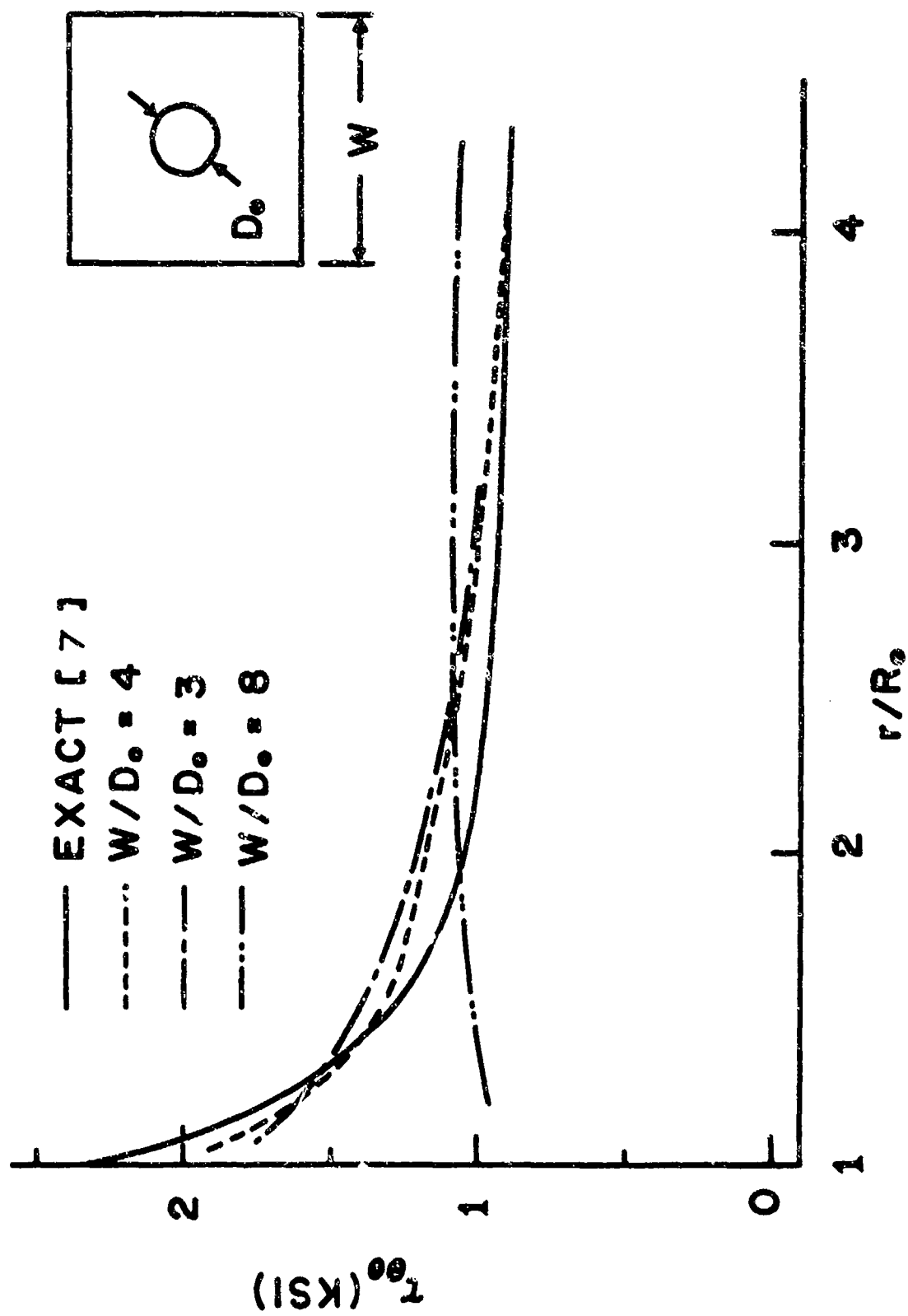


FIG. 16 SENSITIVITY OF HOLE TO SHAPE PARAMETER W/D_0

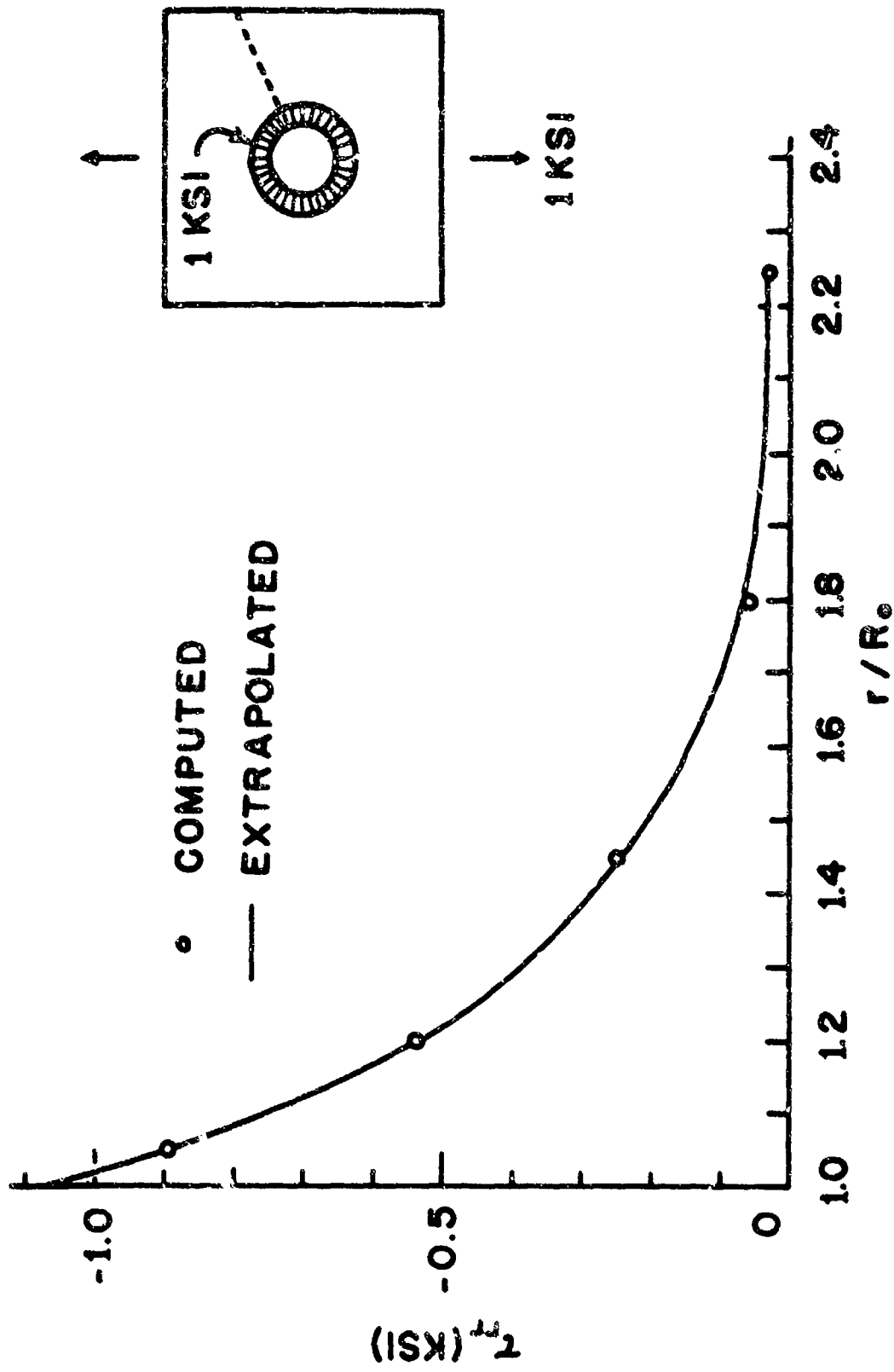


FIG. 17 ABILITY OF HOLE TO ACCEPT AN INTERFERENCE FIT

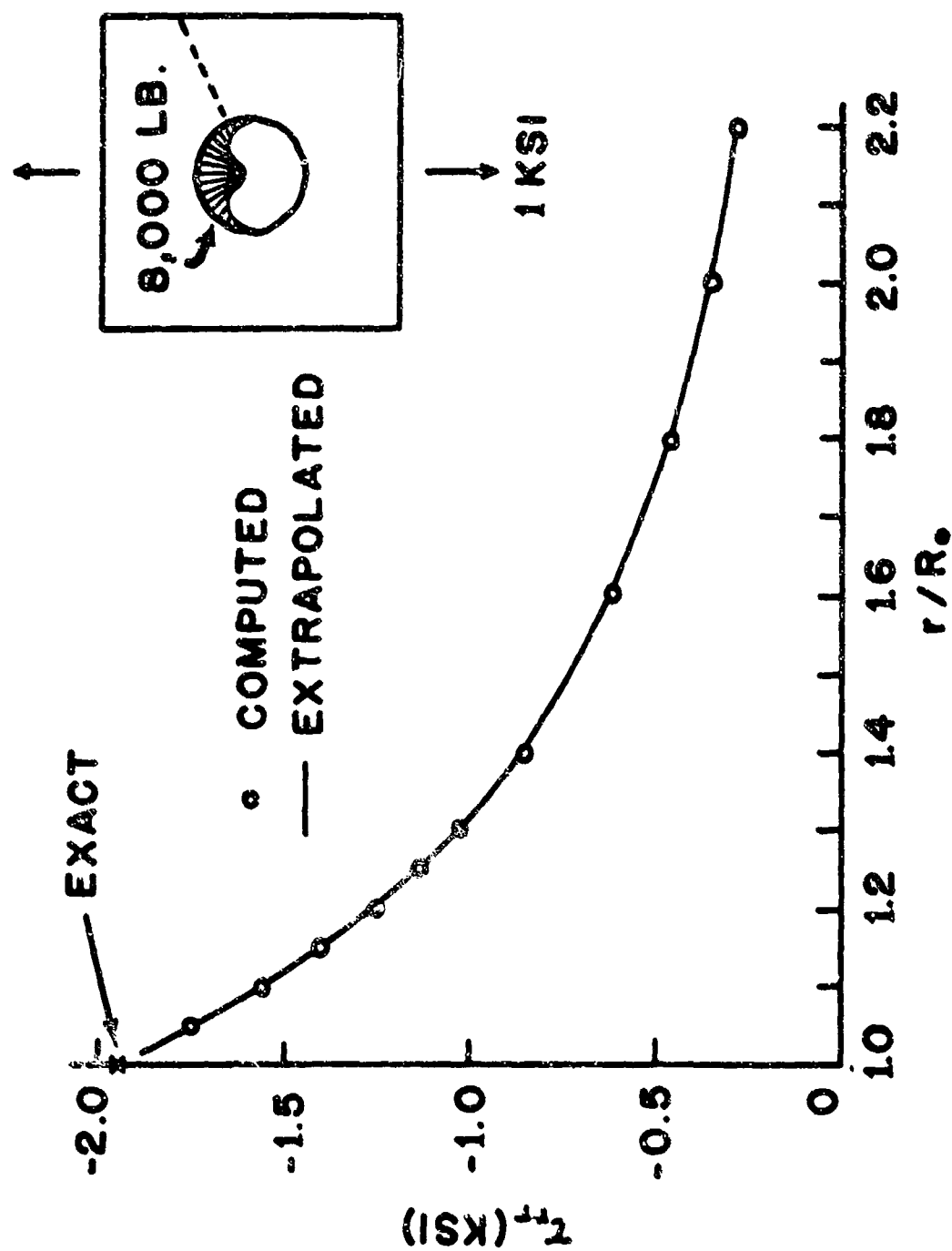


FIG. 18 ABILITY OF HOLE TO ACCEPT A BEARING LOAD

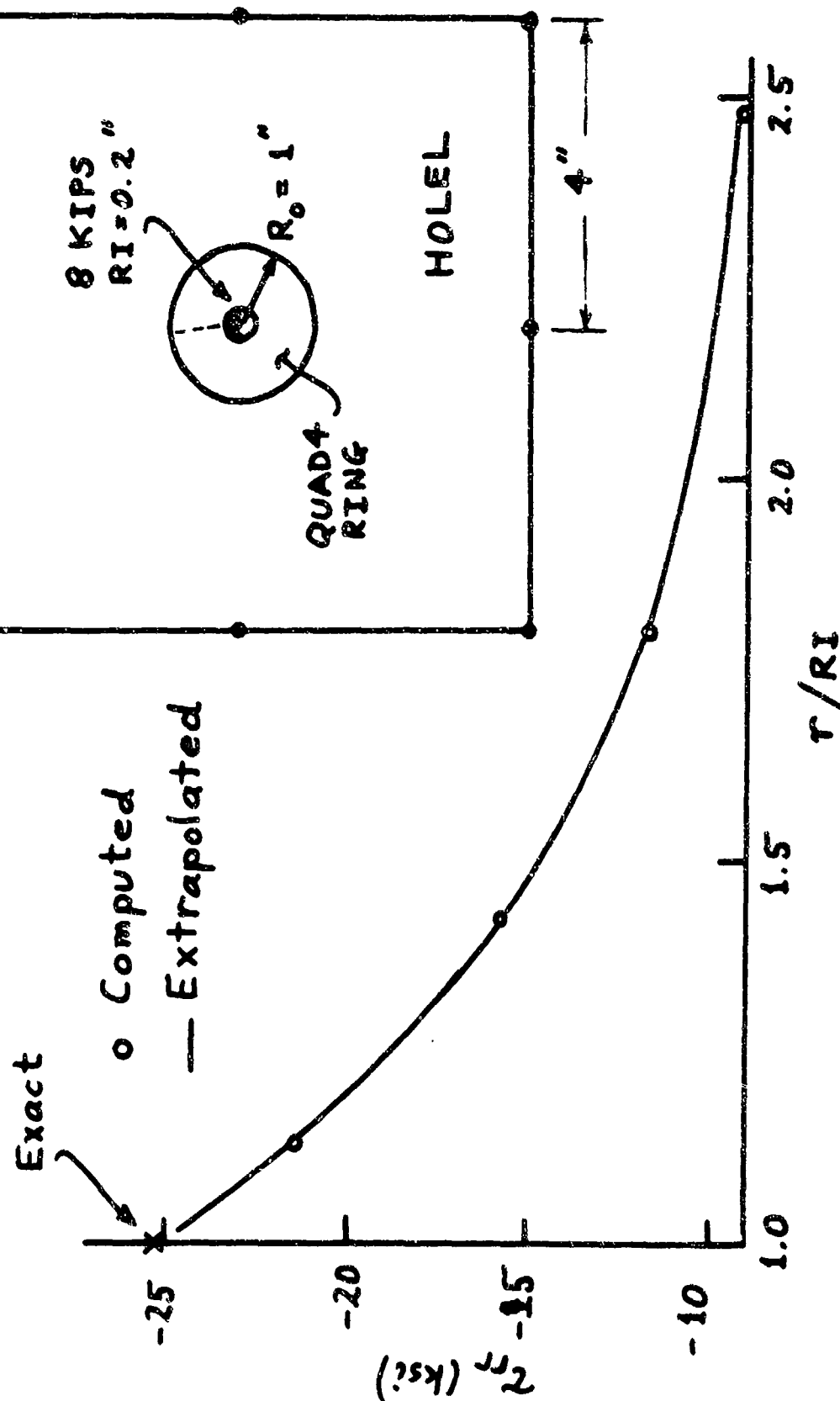


FIG. 19 TEST OF HOLEL WITH INTERIOR RING SUBJECTED TO BEARING LOAD

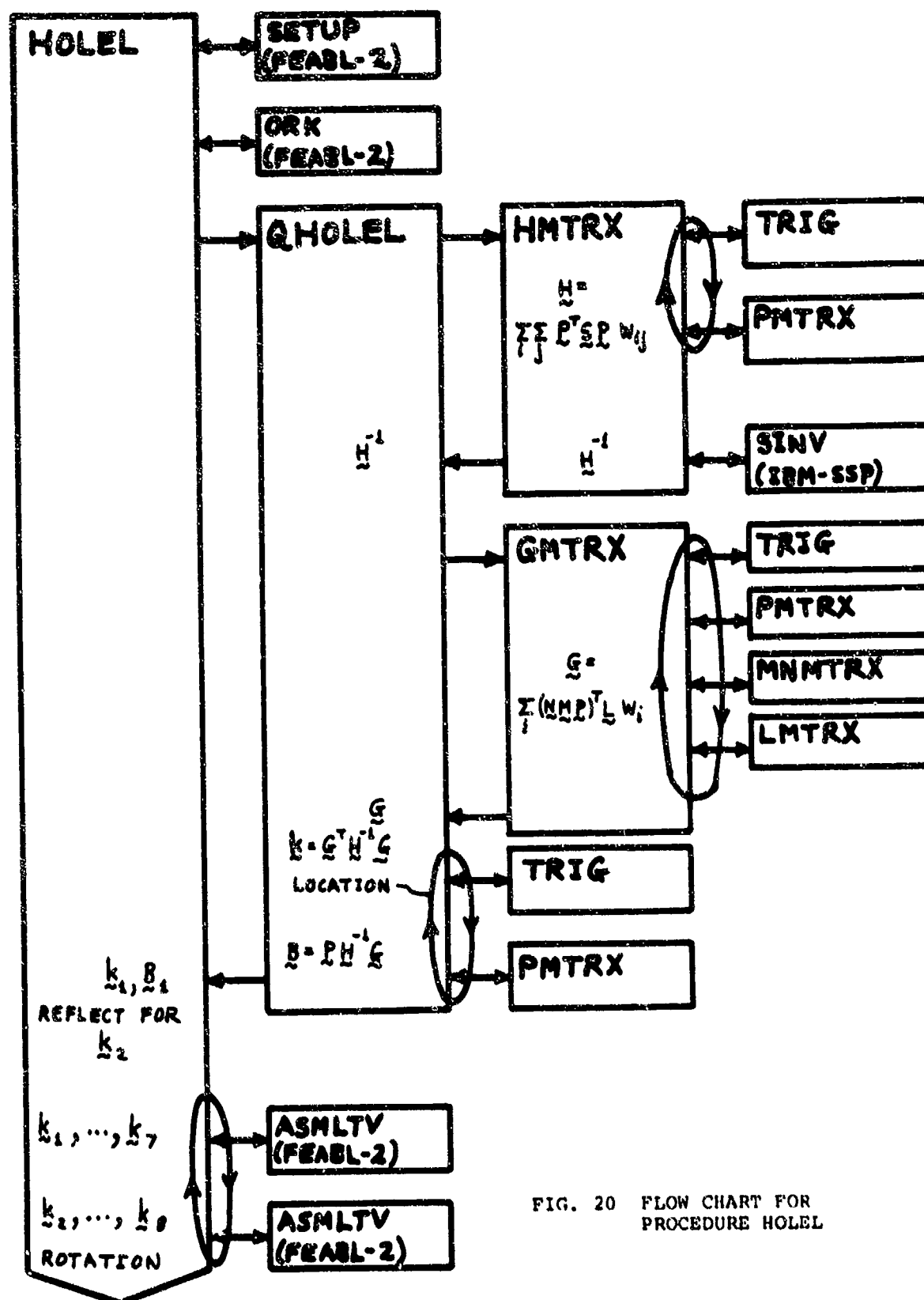


FIG. 20 FLOW CHART FOR
PROCEDURE HOLE

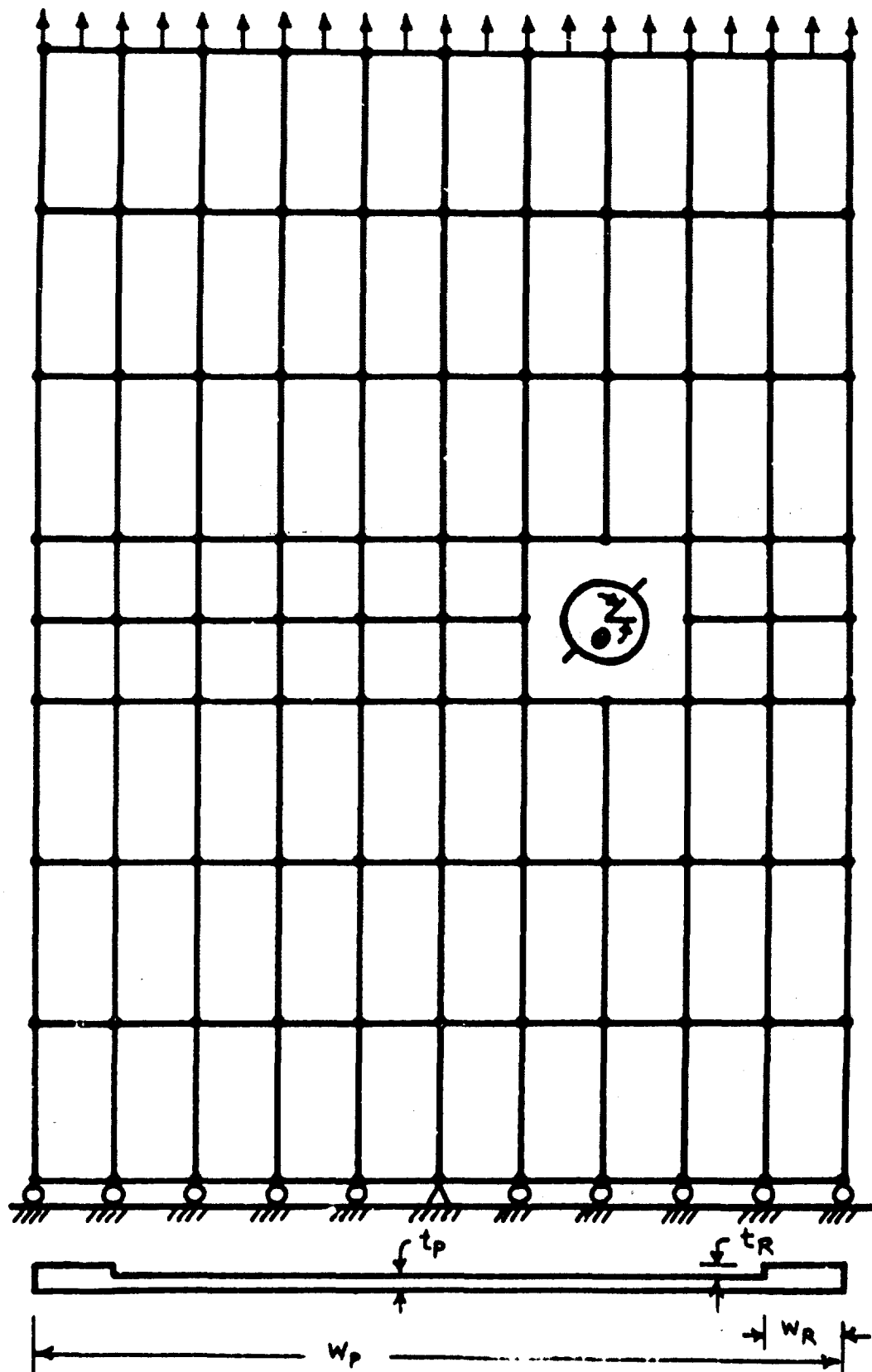


FIG. 21 PANEL PROGRAM STRUCTURE

PANEL PROGRAMS - INPUT DATA CONVENTIONS		ENGINEER STALK/DORRINGER	
NO	PROGRAM FRENCH	DATE 1 APR 1975	SHEET 1 OF 1
CARD			
1	← WIDTH ← LENGTH ← THK ← STIFFT ← PRESS ← RI ← XOFFST (6E10, 7, 25)		
2	← A(1) ← A(2) ← HPOS(1) HPOS(2) (2E10, 3, 215)		
3	← E ← V ← (2E10, 3)		
4	← K71 ← K72 ← K93 ← (315)		

FIG. 22 INPUT DATA CONVENTIONS FOR PANEL PROGRAM

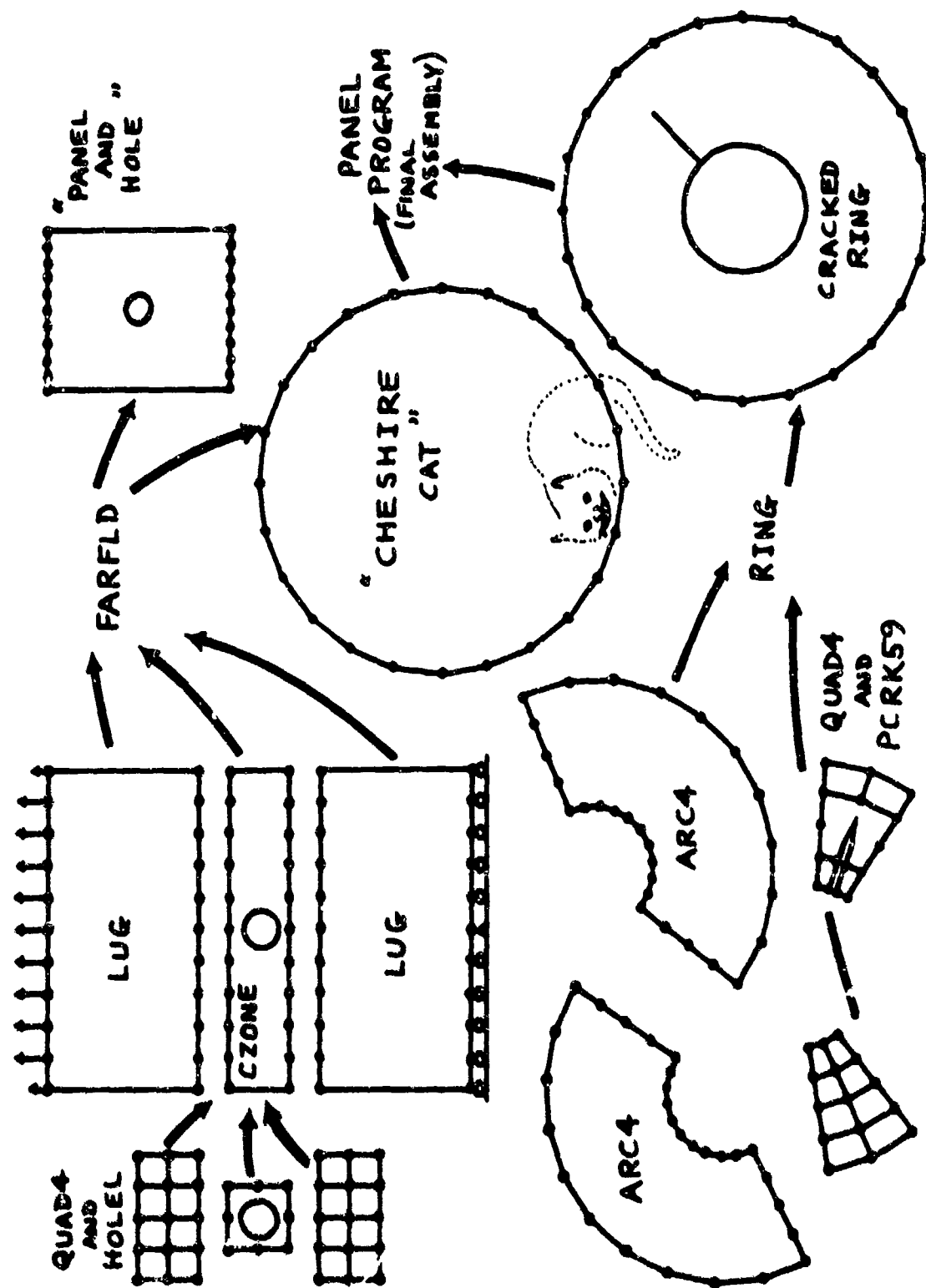
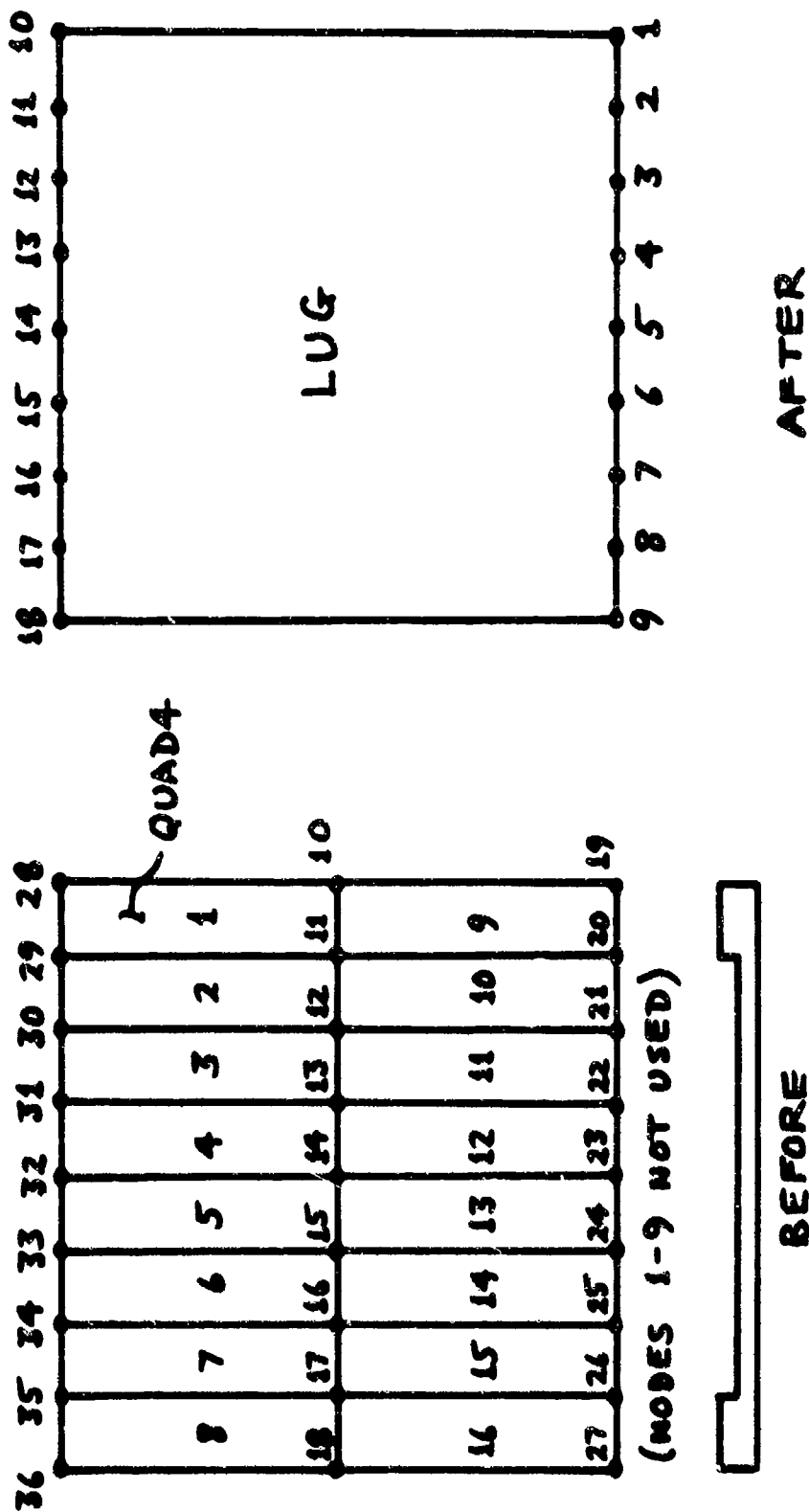


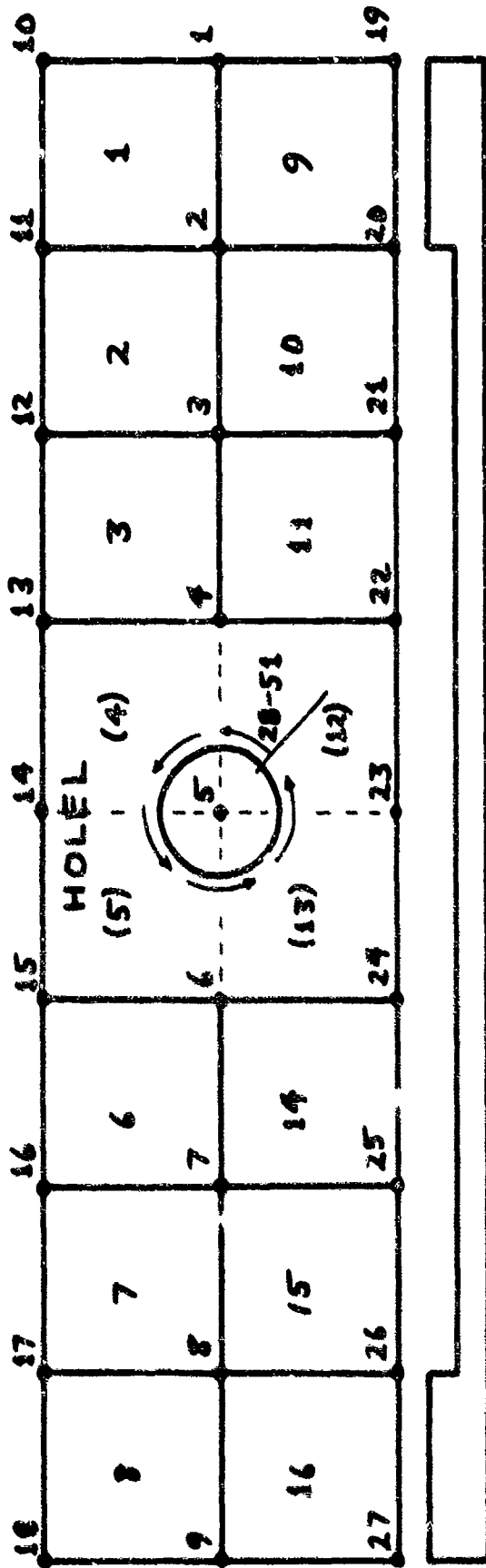
FIG. 23 HIERARCHY OF SUBSTRUCTURED COMPONENTS USED IN PANEL PROGRAM



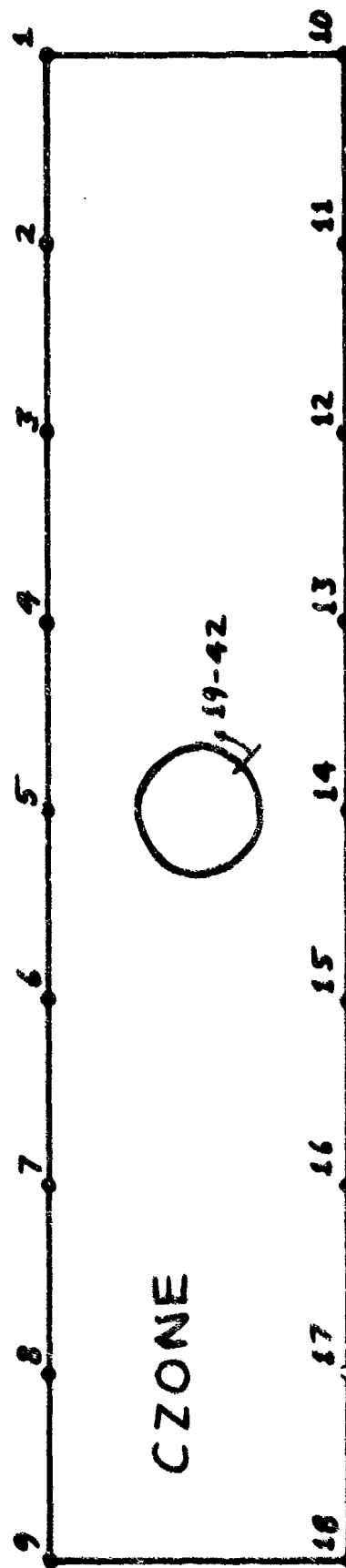
STATIC CONDENSATION
 EXAMPLE WITH 8 ELEMENTS ACROSS PANEL

FIG. 24 LUG SUBSTRUCTURE NUMBERING CONVENTIONS

BEFORE



AFTER STATIC CONDENSATION



EXAMPLE WITH 8 ELEMENTS ACROSS PANEL

FIG. 25 CZONE SUBSTRUCTURE NUMBERING CONVENTIONS

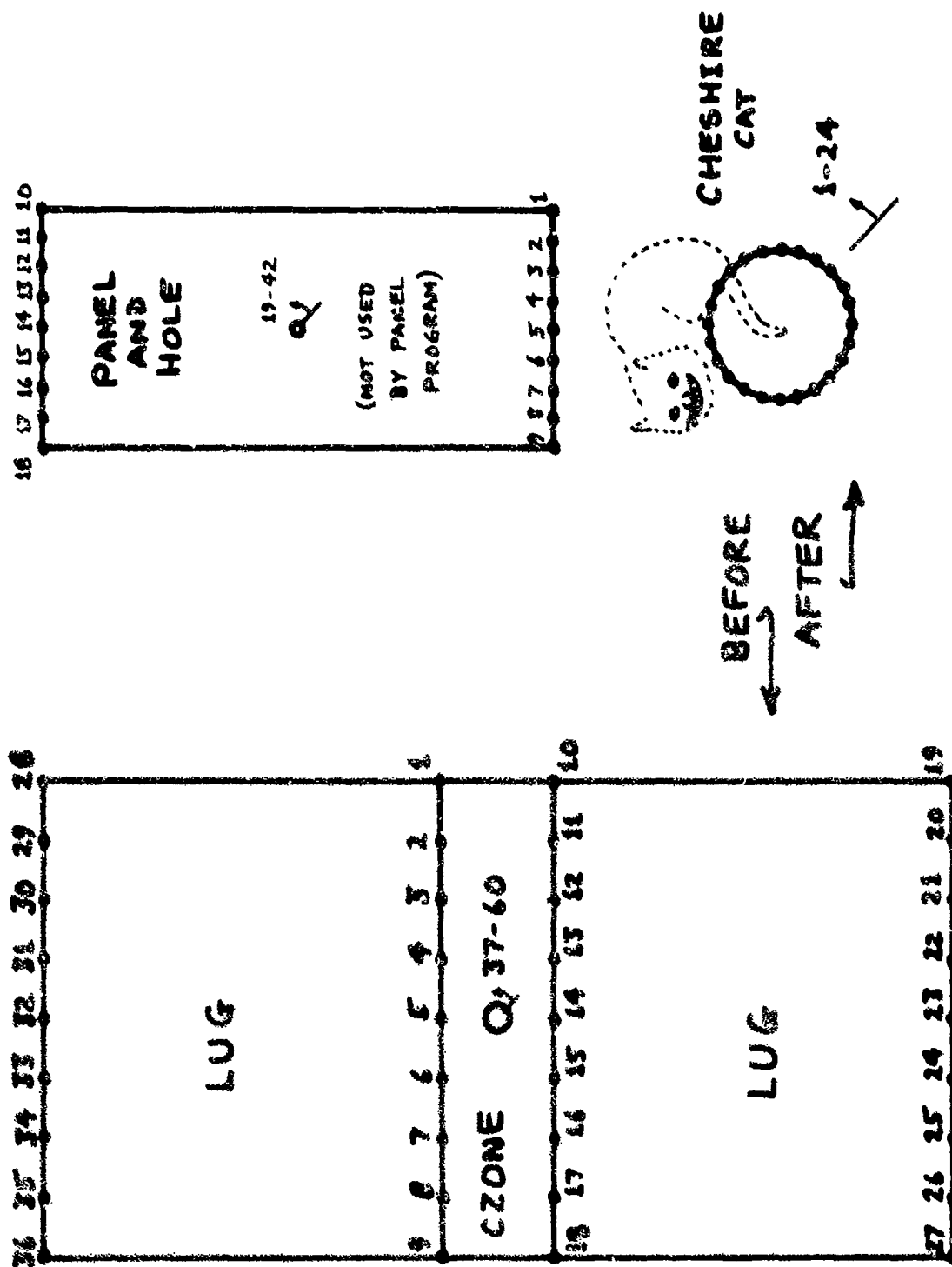


FIG. 24. PANEL SUBSTRUCTURE NUMBERING CONVENTIONS

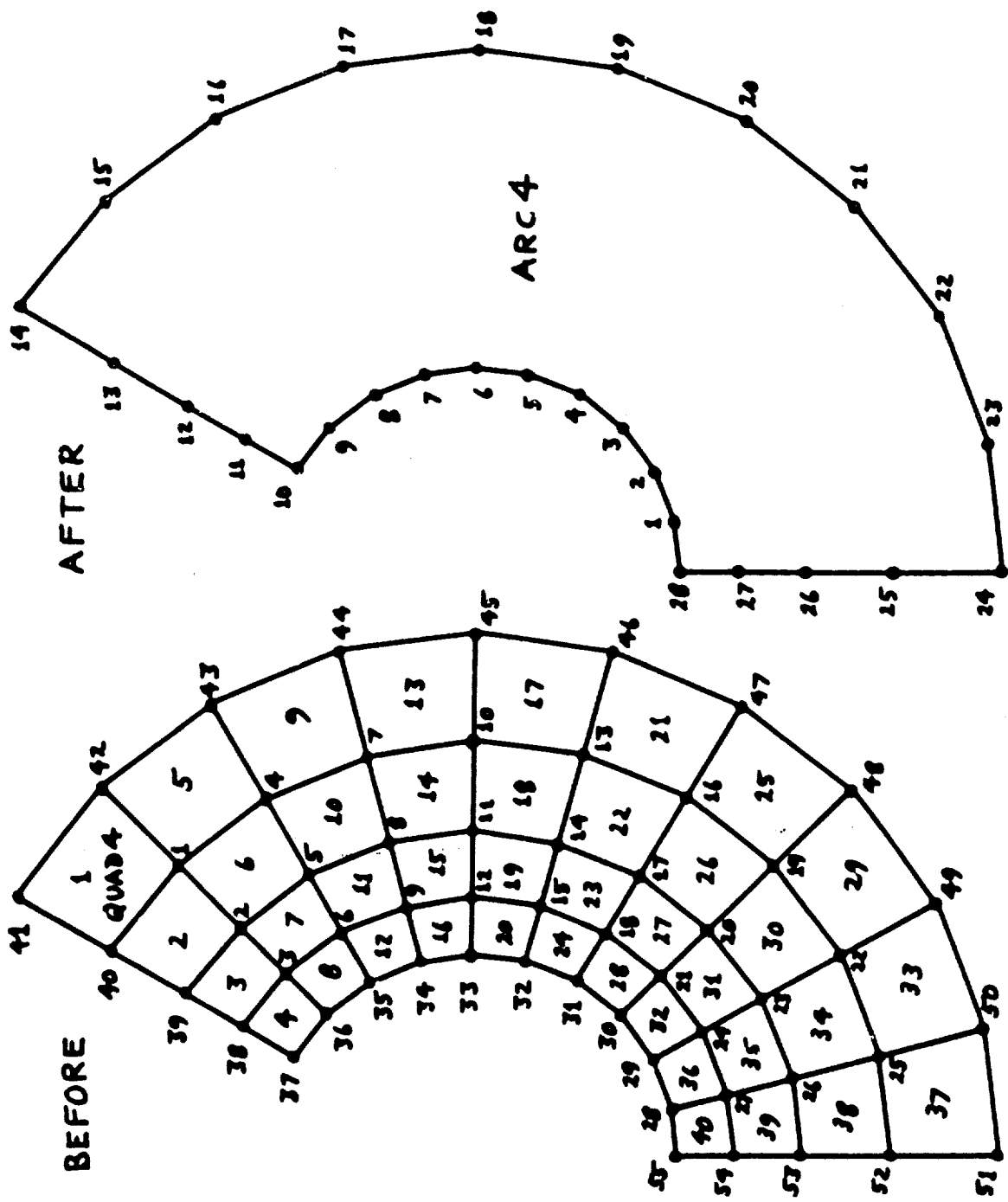
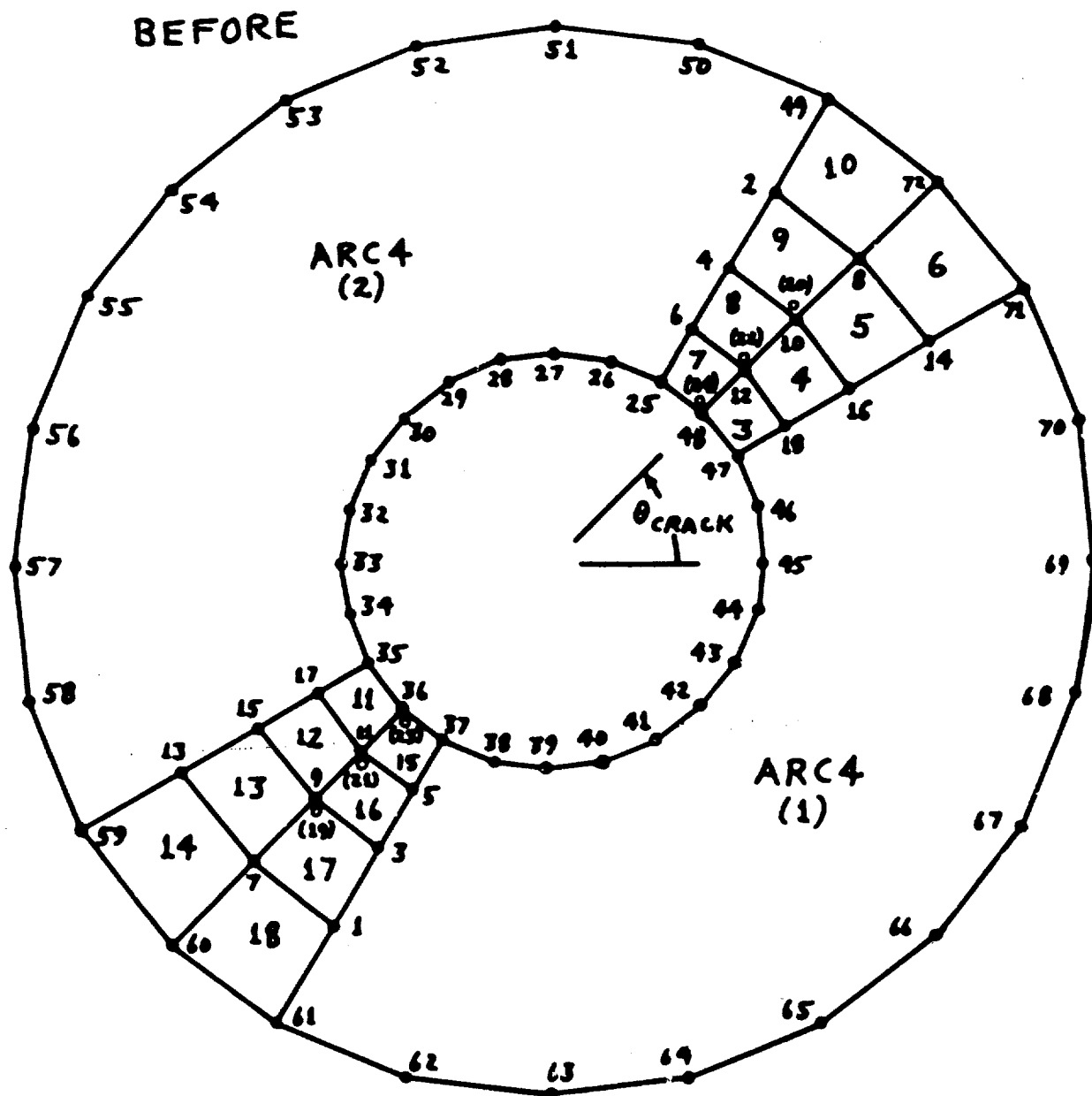


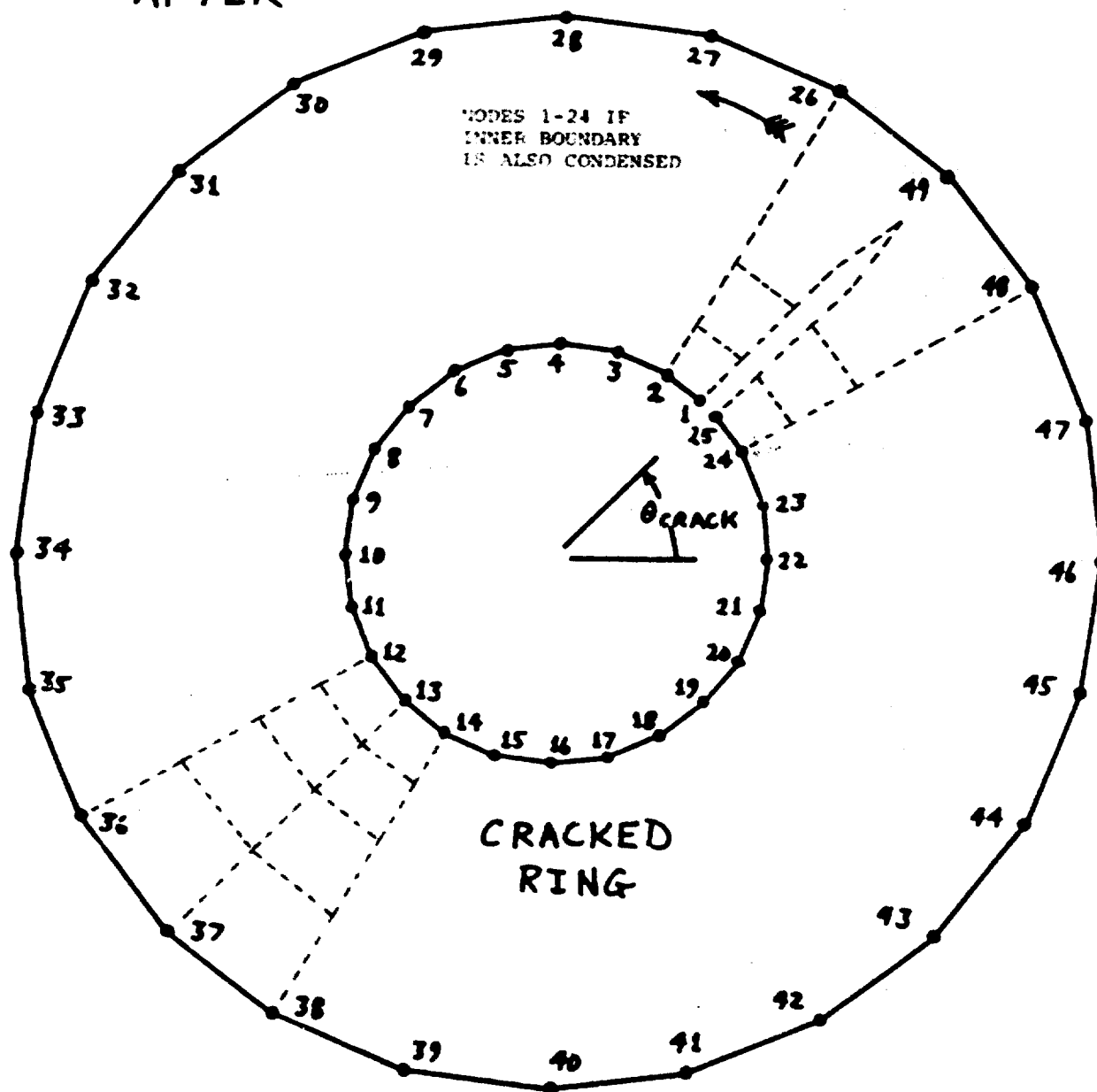
FIG. 27 ARC4 SUBSTRUCTURE NUMBERING CONVENTIONS



NODES (19)-(24) USED ONLY WHEN CRACK IS PRESENT
 PERK59 ELEMENTS ARE 19 (CRACK 1) AND 20 (CRACK 2)

FIG. 26 RING SUBSTRUCTURE CONVENTIONS (BEFORE CONDENSATION)

AFTER



EXAMPLE WITH $\lambda(2) = 0.5$

FIG. 29 RING SUBSTRUCTURE CONVENTIONS (ONE CRACK)

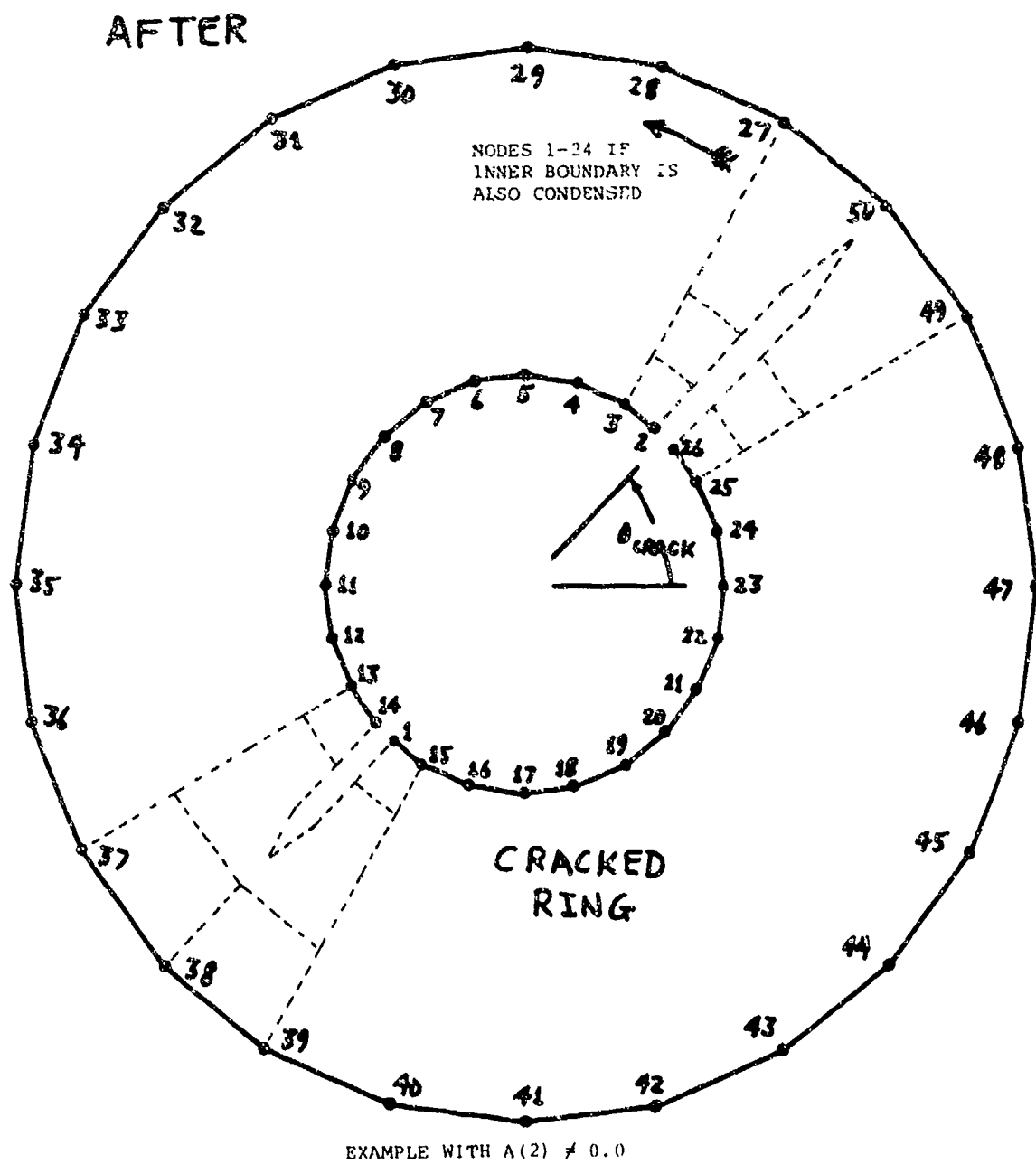


FIG. 30 RING SUBSTRUCTURE CONVENTIONS (TWO CRACKS)

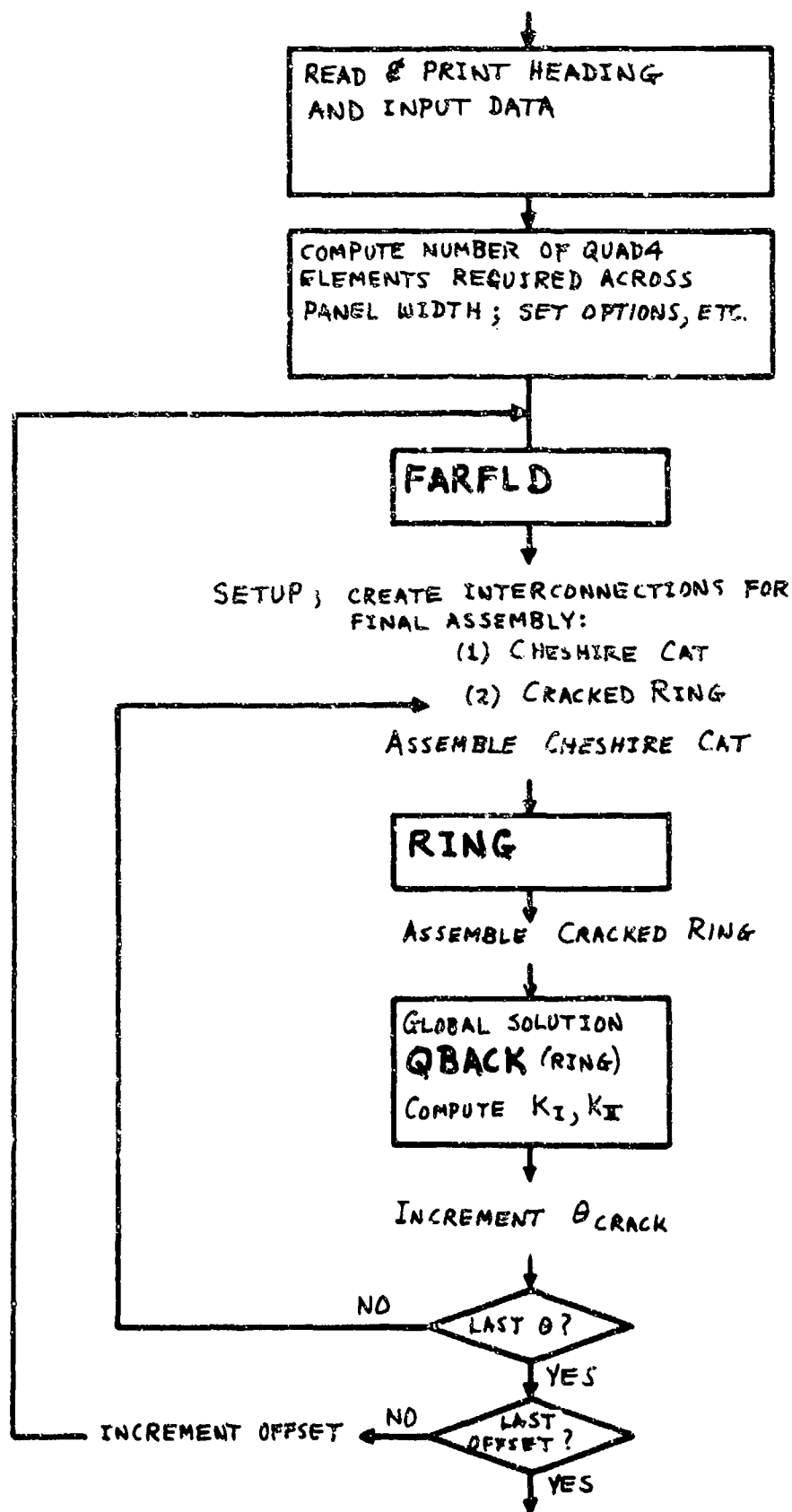


FIG. 31 EXECUTIVE FLOW CHART FOR PANEL PROGRAM

CONTRACT R1739

PROBLEM NO. 3

LEFT SIDE STIFFENED

INPUT DATA

PLATE GEOMETRY:

PLATE WIDTH= 0.40000E+01
PLATE LENGTH= 0.10000E+02
PLATE THICKNESS= 0.10000E+01
PLATE STIFFNESS FACTOR= 0.500
FAR FIELD LOADING (FSI)= 0.10000E+04
RADIUS OF HOLE= 0.12500E+00
HOLE OFFSET INDICATOR= 1(=0, HOLE REMAINS CENTERED
=-1, HOLE MOVED TO THE RIGHT
=+1, HOLE MOVED TO THE LEFT)

CRACK DATA:

CRACK NO. 1 : CRACK LENGTH= 0.12500E+00
INITIAL CRACK POSITION= 1(0.0 DEGREES)
FINAL CRACK POSITION= 13(180.000 DEGREES)

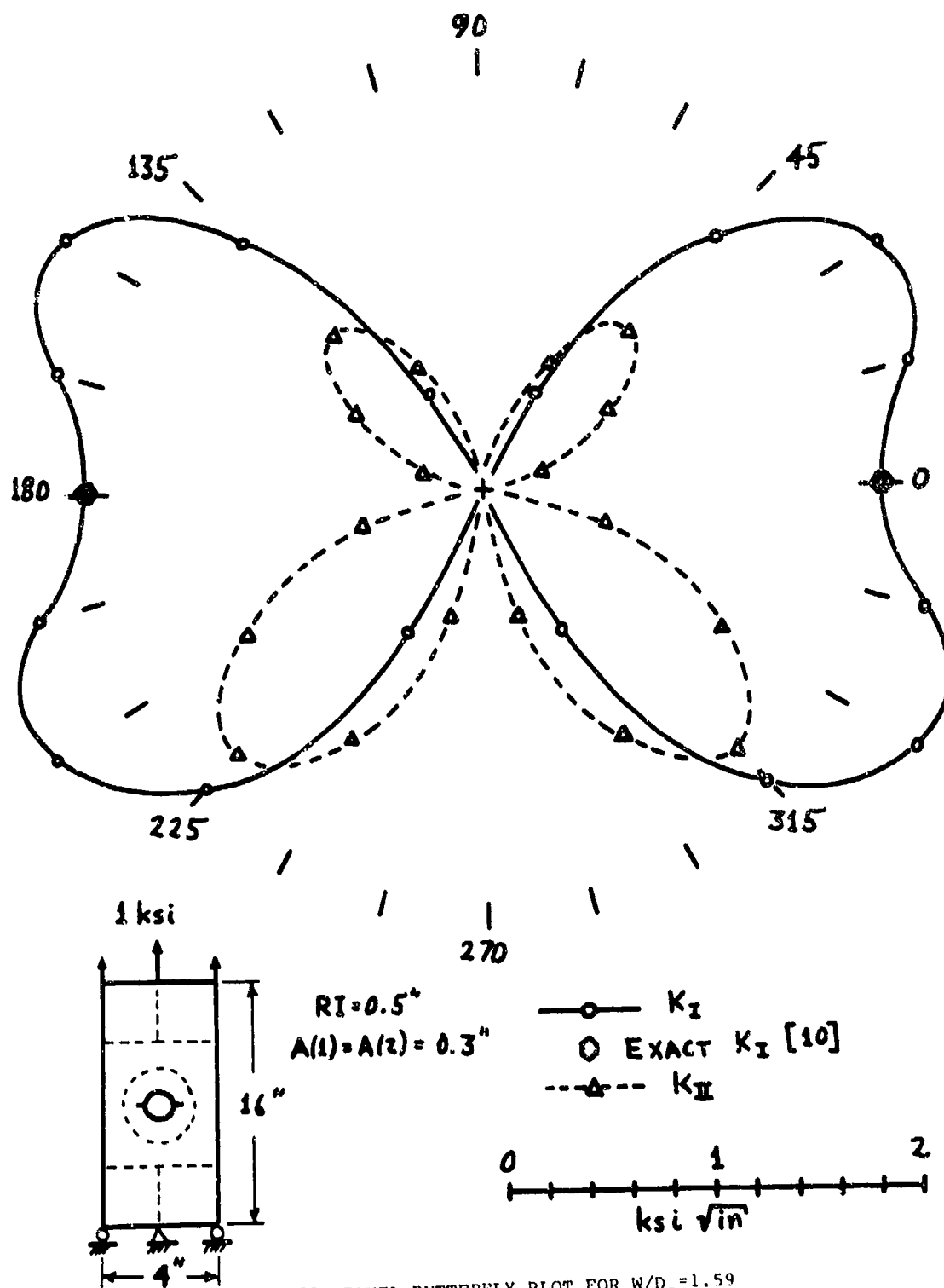
CRACK NO. 2 : CRACK LENGTH= 0.12500E+00
INITIAL CRACK POSITION= 1(180.000 DEGREES)
FINAL CRACK POSITION= 13(360.000 DEGREES)

MATERIAL PROPERTIES:

YOUNG'S MODULUS (E)= 0.10000E+08
POISSON'S RATIO= 0.30000E+00

CRACK NO. 1	ANGLE= 0.0	KI= 0.87369E+03	KII= 0.11424E+00
CRACK NO. 2	ANGLE= 180.000	KI= 0.87216E+03	KII= 0.45826E+00
CRACK NO. 1	ANGLE= 15.000	KI= 0.87397E+03	KII= 0.35178E+03
CRACK NO. 2	ANGLE= 195.000	KI= 0.87177E+03	KII= 0.35054E+03
CRACK NO. 1	ANGLE= 30.000	KI= 0.82898E+03	KII= 0.58763E+03
CRACK NO. 2	ANGLE= 210.000	KI= 0.82647E+03	KII= 0.58543E+03
CRACK NO. 1	ANGLE= 45.000	KI= 0.60824E+03	KII= 0.58508E+03
CRACK NO. 2	ANGLE= 225.000	KI= 0.60659E+03	KII= 0.58246E+03
CRACK NO. 1	ANGLE= 60.000	KI= 0.27609E+03	KII= 0.44344E+03
CRACK NO. 2	ANGLE= 240.000	KI= 0.27596E+03	KII= 0.44213E+03

FIG. 32 SAMPLE OUTPUT FROM PANEL PROGRAM



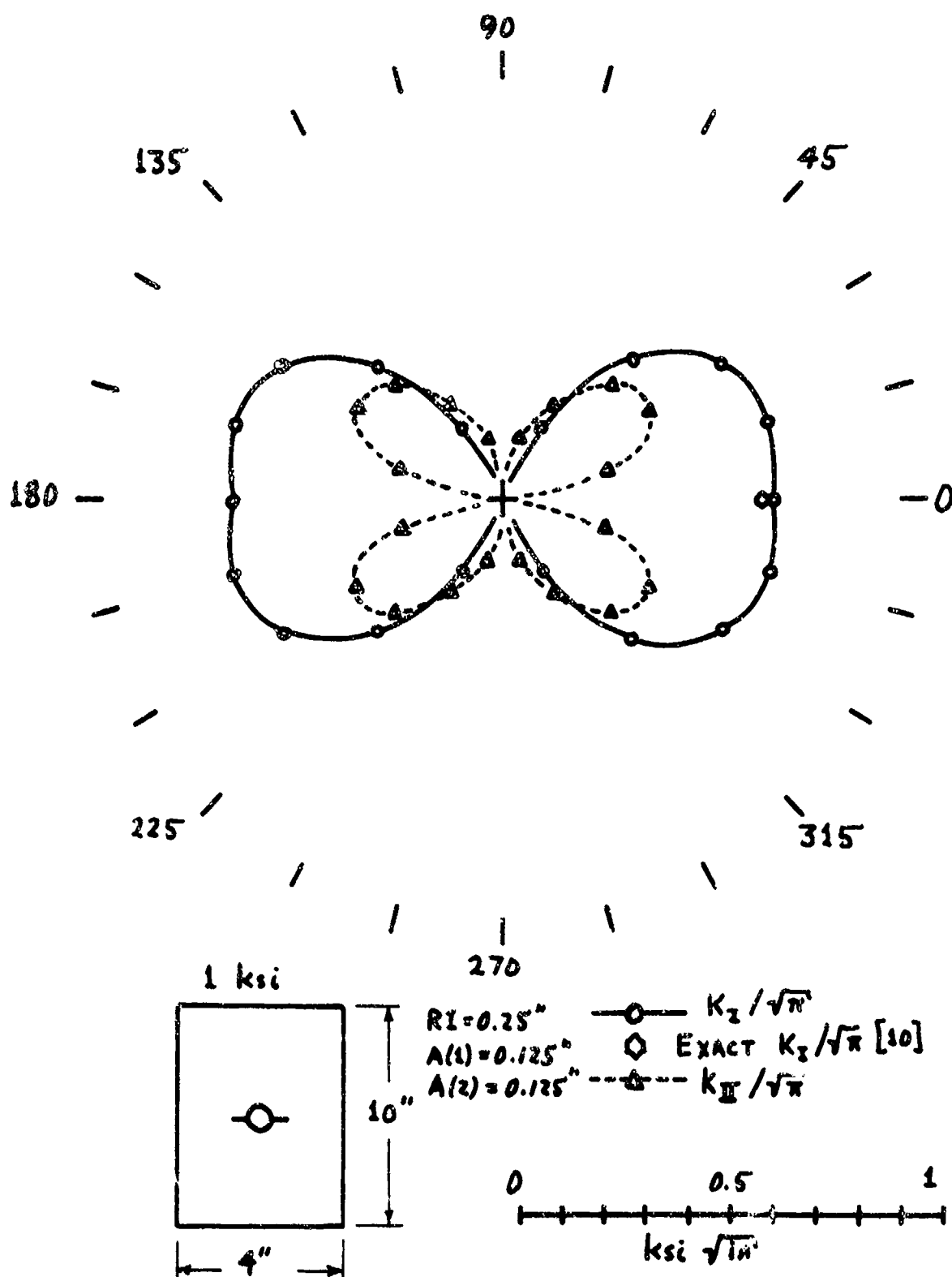


FIG. 34 PANEL BUTTERFLY PLOT FOR $W/D_0 = 3.17$

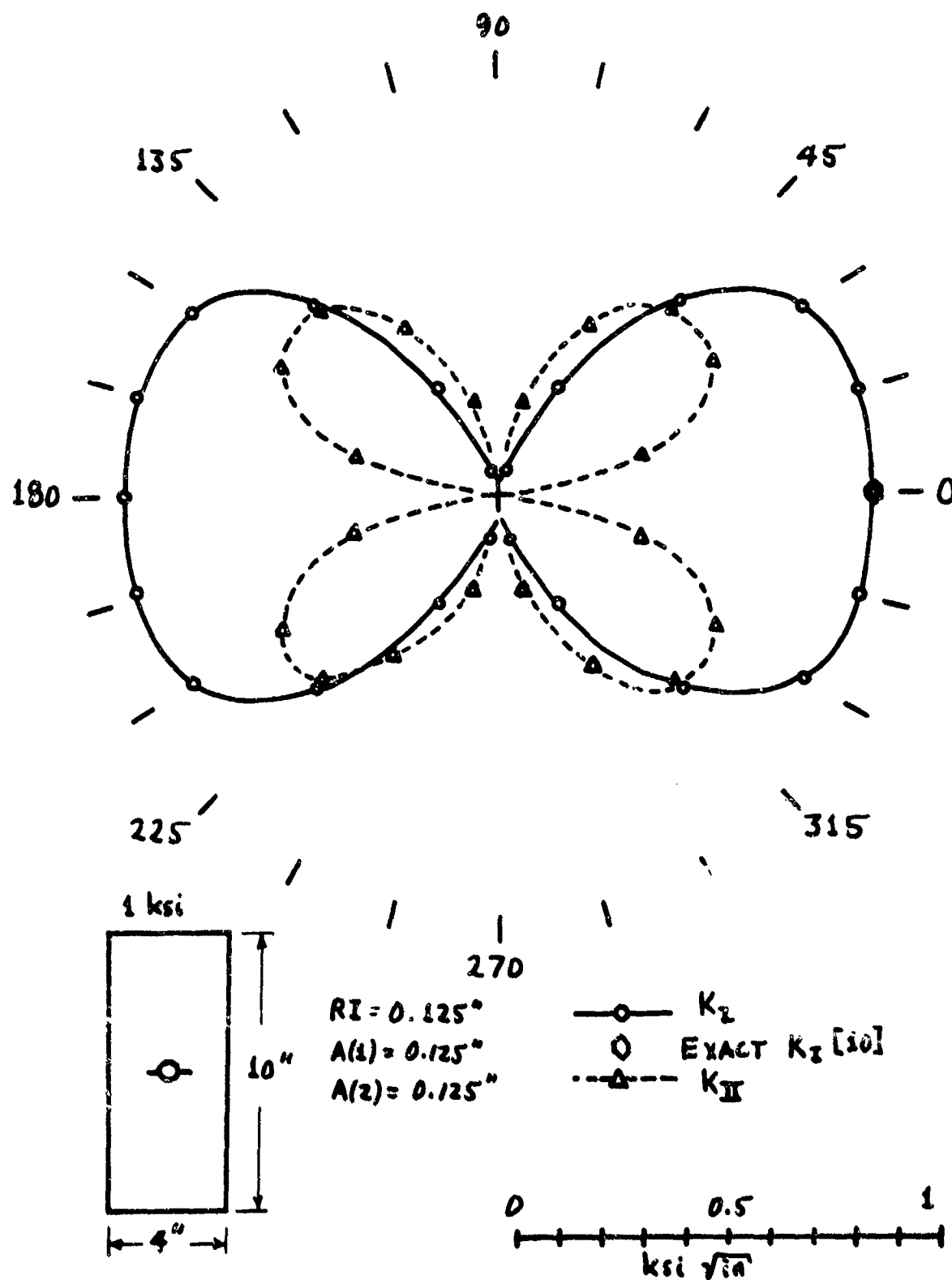


FIG. 35 PANEL BUTTERFLY PLOT FOR $W/D_0 = 6.35$

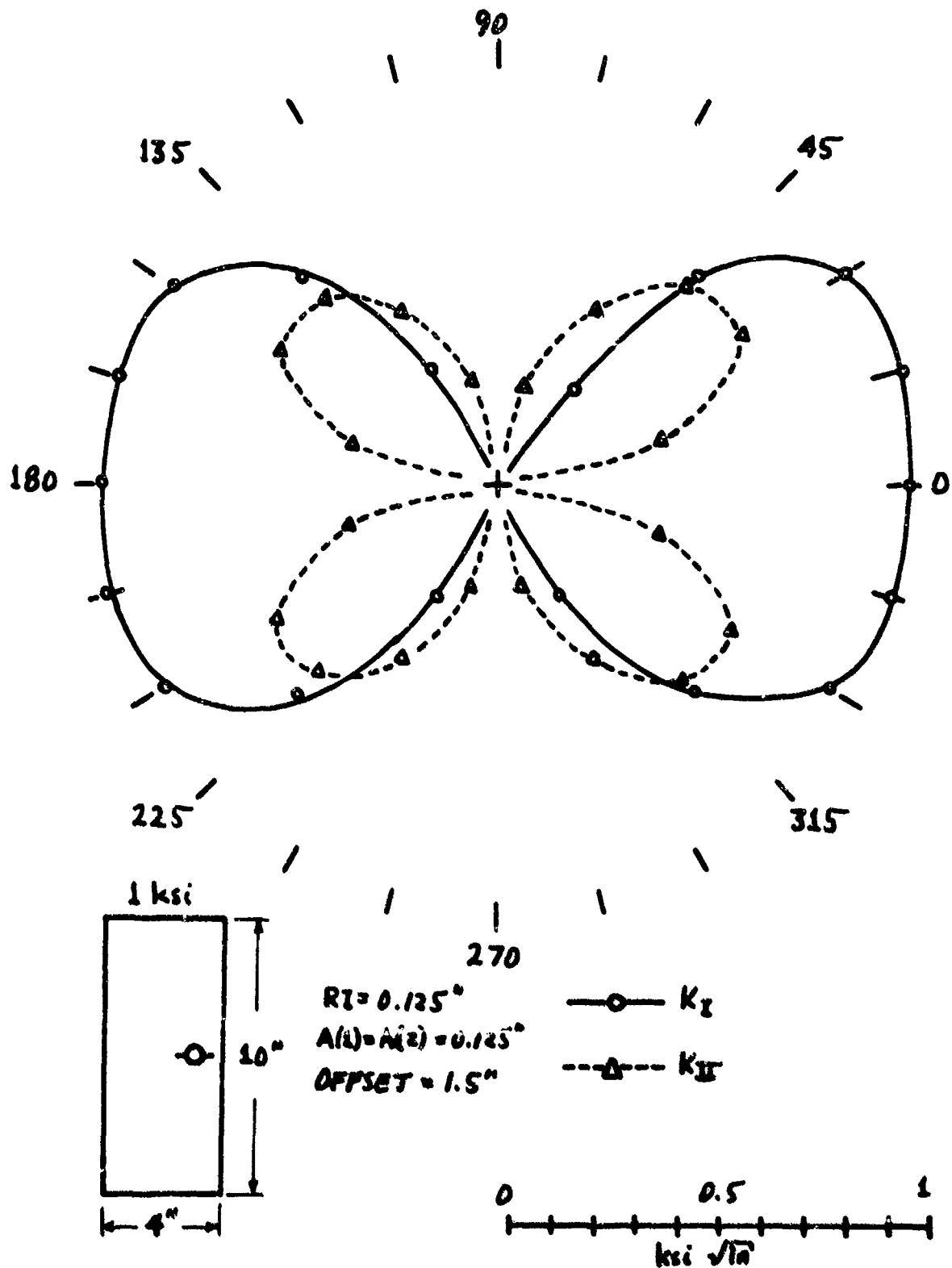


FIG. 36 BUTTERFLY PLOT FOR PANEL WITH $W/D_o = 6.35$ AND FASTENER HOLE OFFSET 1.5 INCHES TO RIGHT

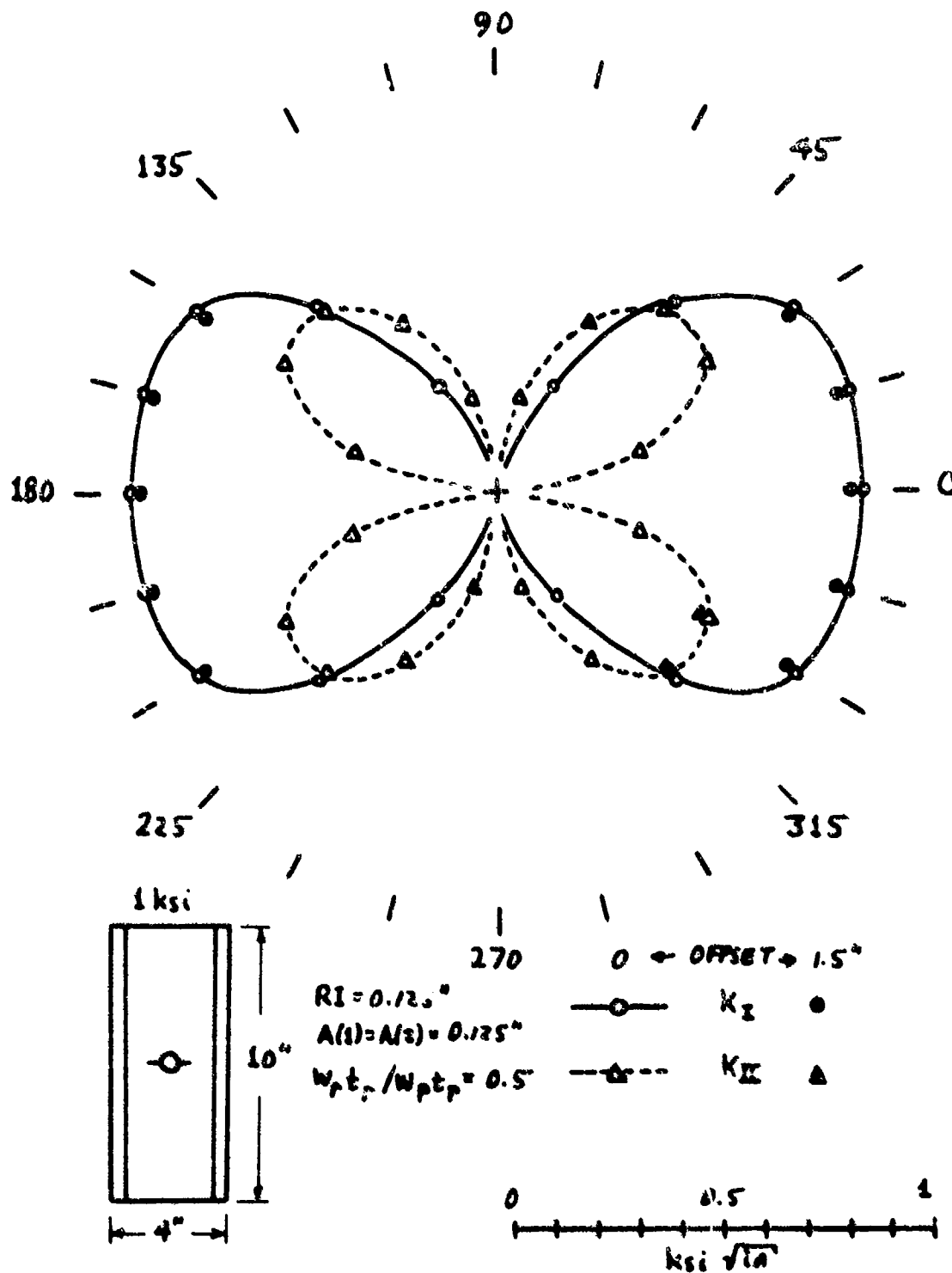


FIG. 12 BUTTERFLY PLOT FOR SYMMETRICALLY STIFFENED PANEL WITH STIFFENER FACTOR = 0.5

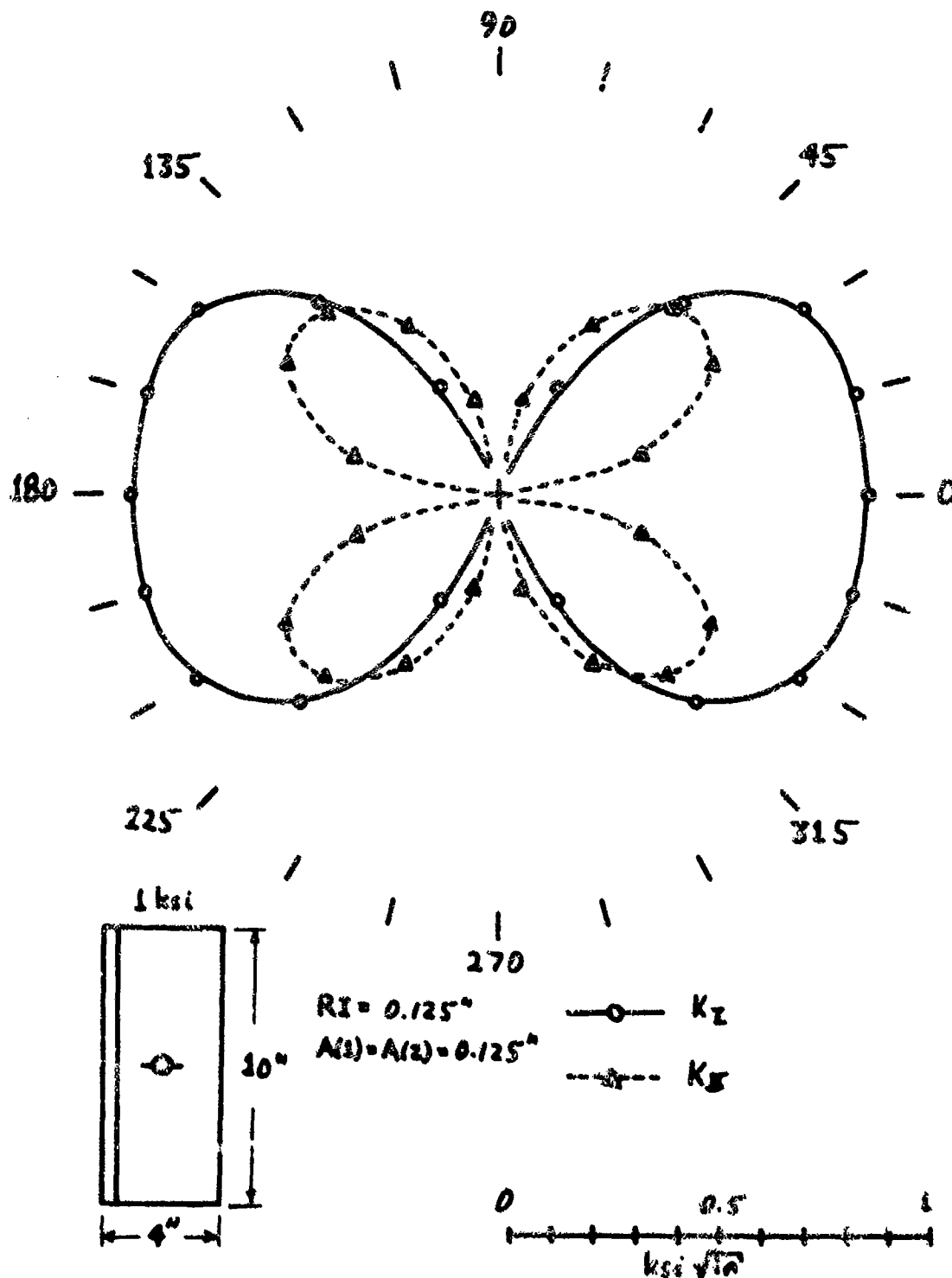


FIG. 38 BUTTERFLY PLOT FOR PANEL WITH LEFT EDGE STIFFENED
(FACTOR = 0.5) AND HOLE CENTERED

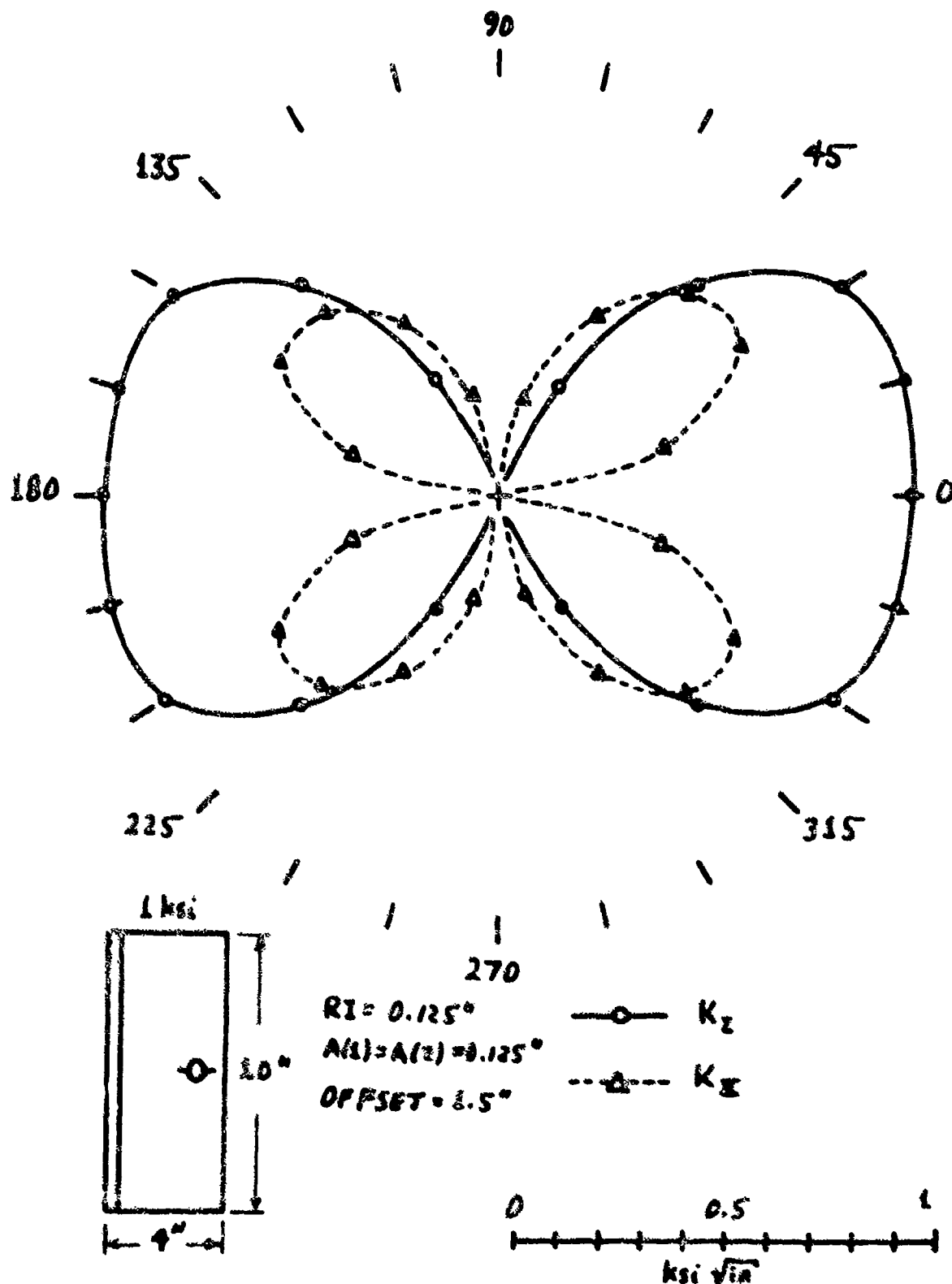


FIG. 19 BUTTERFLY PLOT FOR PANEL WITH LEFT EDGE STIFFENED AND HOLE OFFSET 1.5 INCHES TO RIGHT

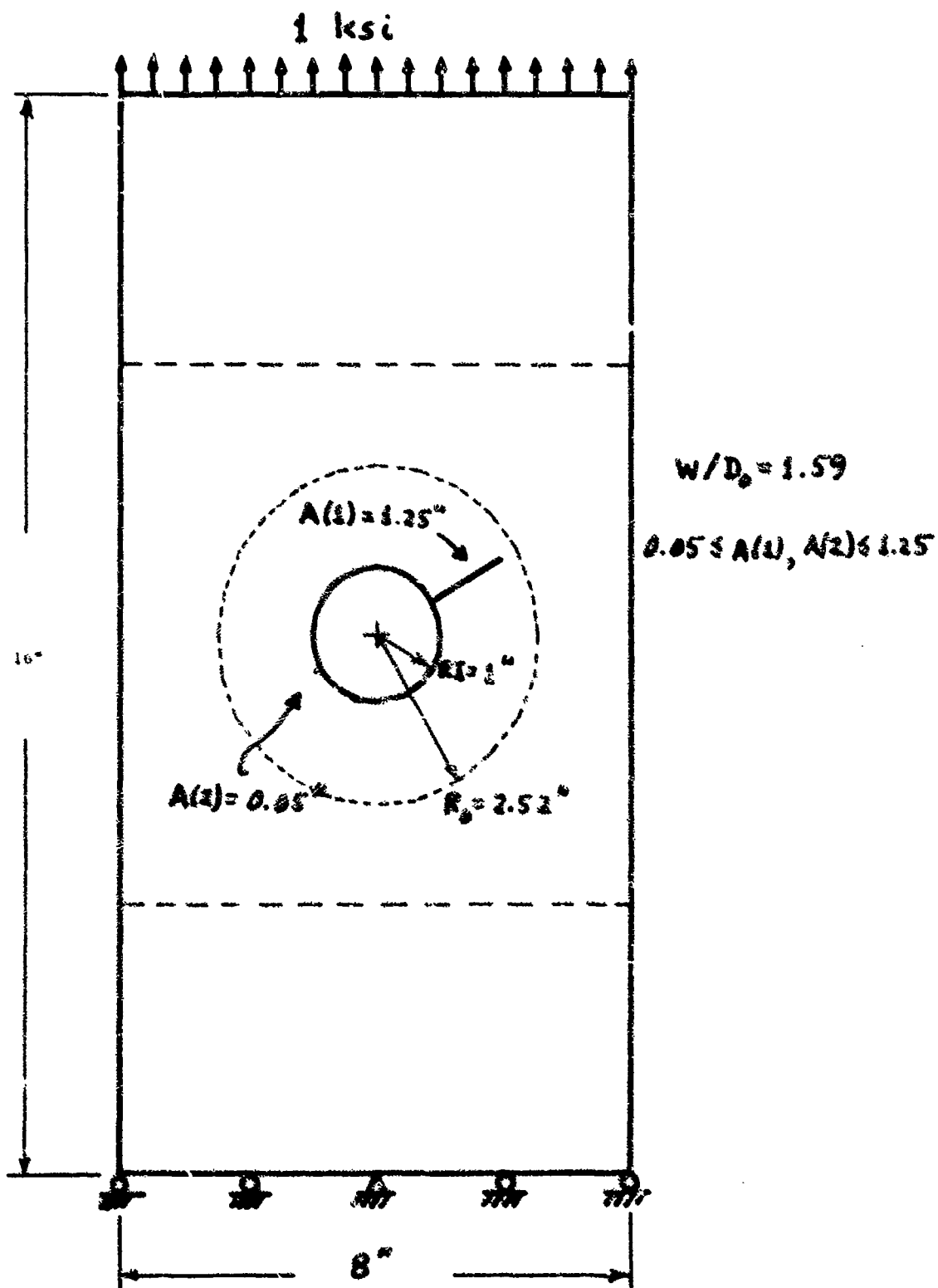


FIG. 40 PANEL DIMENSIONS FOR TEST OF SENSITIVITY TO a/R_1

Appendix A

ALGORITHM FOR TRANSFERRING A SUBSTRUCTURE TO RESERVE STORAGE

Assume that a standard FLABL-2 assembly has been executed, and that the assembled structure has been statically condensed with subroutine STACON. The objective is to transfer the condensed stiffness matrix $K_{BB}^{(c)}$ and force $\bar{Q}_B^{(c)}$ from the FEABL DATA vector (designated by RNAME, INAME) to reserve storage. Assume further that $K_{EB}^{(c)}$ is almost fully populated. Therefore, $K_{BB}^{(c)}$ is to be inflated to a fully populated, lower-triangle-vector-stored matrix during the transfer process. For this purpose, vectors STORK and STORQ are dimensioned in the MAIN program to at least $N(N+1)/2$ and N words respectively, where N is the total number of boundary (uncondensed) degrees of freedom remaining. Vector STORK will receive $K_{BB}^{(c)}$, while STORQ receives $\bar{Q}_B^{(c)}$.

Other variables appearing in the algorithm play the following roles. IZERO is the first uncondensed degree of freedom, i.e., $IZERO = NIO+1$. I and J are loop indices which control progress through $K_{BB}^{(c)}$ and $\bar{Q}_B^{(c)}$ in the DATA vector: I for the rows and J for the columns. If a row is encountered for which the band margin of $K_{BB}^{(c)}$ lies to the right of column IZERO, then some leading zeros must be inserted in the corresponding row in STORK. LN2COL is used to compute and compare the band margin. II is used to hold the variable-bandwidth address information required to locate

stiffnesses in the DATA vector belonging to ROW I. M, N and MN are used to trace the lower-triangle address in vector STORK. IKOUNT, ILNZ and IQ are FEABL address control parameters located in the BEGIN labelled COMMON area. NDT is the total number of degrees of freedom in the assembled structure (boundary plus interior), available in the SIZE labelled COMMON area.

The transfer algorithm begins immediately after subroutine STACON has been executed:

```

      .
      .
      .
      IZERO = NID+1
      M=0
      DO 20 I=IZERO, NDT
      II=INAME(IKOUNT+I-1)
      M=M+1
      STORQ(M)=RNAME(IQ+I-1)
      N=0
      DO 10 J=IZERO,I
      N=N+1
      MN=(M*(M-1))/2+N
      STORK(MN)=0.0
      LNZCOL=INAME(ILNZ+I-1)
      IF(LNZCOL-J) 5,5,10
5      STORK(MN) = RNAME(II+J)
10     CONTINUE
20     CONTINUE
      .
      .
      .

```


APPENDIX B

```

SUBROUTINE HOLEL(COORD,THK,S,RI,RSS,ISS,B)
*****
C SUBROUTINE HOLEL GENERATES A RECTANGULAR FINITE ELEMENT SUBSTRUCTURE
C WITH A ROUND HOLE IN ITS CENTER. THERE ARE 8 NODES ON ITS PERIMETER
C AND 24 NODES AROUND THE HOLE
*****
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY
*****
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS
*****
DIMENSION RSS(2097),ISS(2097),ISAVE1(2),ISAVE2(6),ISAVE3(6)
@,COORD(12),S(3,3),V(78),ELQ(12),R(6,3,13),VR(78),I(12,12),
@ELKR(12,12),ELK(12,12),NODEL(6,8)
COMMON/IO/KR,KP,KT,KT2,KT3
COMMON/SIZE/NET,NOT
COMMON/REGIN/IREGIN(6)
COMMON/END/JEND(6)
COMMON/SIZESS/NETSS,NOTSS,NISS
COMMON/REGSS/IRG(6)
DATA PI/3.141593/
DATA NODEL/2,1,9,10,11,12,2,3,15,14,13,12,
@4,3,15,16,17,18,4,5,21,20,19,18,
@6,5,21,22,23,24,6,7,27,26,25,24,
@8,7,27,28,29,30,8,1,9,32,31,30/
DO 130 I=1,12
DO 130 J=1,I
T(I,J)=0.0
T(J,I)=0.0
130 T(J,I)=0.0
C SAVE CALLING PROGRAM FEARL CONTROLS
ISAVE1(1)=NET
ISAVE1(2)=NOT
DO 10 I=1,6
ISAVE2(I)=IREGIN(I)
ISAVE3(I)=IEND(I)
PI4=PI/4.0
PI2=PI/2.0
10

```



```

NET=8
NPT=64
CALL SETUP(2097,9,104,RSS,ISS)
C ESTABLISH ASSEMBLY LIST
  IMASTR=IBEGIN(4)
  LMASTR=IEND(4)
  DO 40 N=1,NET
    NM1=N-1
    IPNTR=IMASTR+NFT+NM1*12
    ISS(IMASTR+NM1)=IPNTR
    DO 40 I=1,6
      IADD=IPNTR+I*2-1
      IDOF=NODEL(I,N)*2
      ISS(IADD-1)=IDOF-1
      ISS(IADD)=IDOF
      IF(KTI.EQ.KW) GO TO 707
      WRITE(N*,5001)(ISS(I),I=IMASTR,LMASTR)
5001 FORMAT(30H MASTER ASSEMBLY LIST: HOLES:./,10H POINTERS:./,16H
      @ ELEMENT 0.0.F.S./,R(1H,1215./))
707 CONTINUE
      CALL ORK(2097,RSS,ISS)
      C DETERMINE NODE/QUADRANT A.SOC.
      XC=0.0
      YC=0.0
      DO 50 I=1,4
        N=I*3-3
        XC=XC+COORD(N+1)/4.0
        YC=YC+COORD(N+2)/4.0
50      DO 60 I=1,4
        N=I*3-3
        COORD(N+1)=COORD(N+1)-YC
        COORD(N+2)=COORD(N+2)-YC
        THETA1=0.0
        IF(COORD(1).GT.0.0.AND.COORD(2).GT.0.0) THETA1=PI2
        IF(COORD(1).LT.0.0.AND.COORD(2).GT.0.0) THETA1=PI
        IF(COORD(1).LT.0.0.AND.COORD(2).LT.0.0) THETA1=PI+PI2
60

```



```

HWD=ABS(COORD(1))
CALL QHOLEL(TM,K,S,PI4,HWD,RI,ELK,ELQ,H)
OBTAIN REFLECTION
T(1,1)=1.0
T(2,2)=-1.0
DO 70 IY=2,12,2
  ITM2=IY-2
  DO 70 I=1,2
    T(ITM2+I,ITM2+I)=T(I,I)
  DO 80 I=1,12
    DO 80 J=1,12
      ELKR(I,J)=0.0
    DO 80 K=1,12
      DO 80 L=1,12
        ELKR(I,J)=ELKR(I,J)+T(K,I)*ELK(K,L)*T(L,J)
TRANSFORM ELEMENTS
THETA =THETA1
NEL1=-1
NEL2=0
DO 110 N=1,4
  NEL1=NEL1+2
  NEL2=NEL2+2
  GO TO (1,2,3,4),N
1  CS=1.0
   SS=0.0
   GO TO 5
2  CS=0.0
   SS=1.0
   GO TO 5
3  CS=-1.0
   SS=0.0
   GO TO 5
4  CS=0.0
   SS=-1.0
   CONTINUE
5  T(1,1)=CS

```

```

HOLE0072
HOLE0073
HOLE0074
HOLE0075
HOLE0076
HOLE0077
HOLE0078
HOLE0079
HOLE0080
HOLE0081
HOLE0082
HOLE0083
HOLE0084
HOLE0085
HOLE0086
HOLE0087
HOLE0088
HOLE0089
HOLE0090
HOLE0091
HOLE0092
HOLE0093
HOLE0094
HOLE0095
HOLE0096
HOLE0097
HOLE0098
HOLE0099
HOLE0100
HOLE0101
HOLE0102
HOLE0103
HOLE0104
HOLE0105
HOLE0106
HOLE0107

```



```

T(1,2)=SS
T(2,1)=-T(1,2)
T(2,2)=T(1,1)
DO 90 IT=2,12,2
ITM2=IT-2
DO 90 I=1,2
DO 90 J=1,2
90 T(ITM2+I,ITM2+J)=T(I,J)
L=0
DO 100 I=1,12
DO 100 J=1,1
L=L+1
V(L)=0.0
VR(L)=0.0
DO 100 K=1,12
DO 100 LL=1,12
V(L)=V(L)+T(K,I)*ELK(K,LL)*T(LL,J)
100 VR(L)=VR(L)+T(K,I)*ELK(K,LL)*T(LL,J)
CALL ASMLTV(NEL1,12,VR,FLO,RSS,ISS)
CALL ASMLTV(NEL2,12,V,FLO,RSS,ISS)
110 THETA=THETA+PI?
C RETURN CALLING PROGRAM FFARL CONTROLS TO THEIR ORIGINAL STORAGE
NETSS=NET
NDTSS=NDT
NIDSS=0
NET=ISAVE1(1)
NDT=ISAVE1(2)
DO 120 I=1,6
IBG(I)=IBEGIN(I)
IBEGIN(I)=ISAVE2(I)
120 IEND(I)=ISAVE3(I)
RETURN
END
HOLE0108
HOLE0109
HOLE0110
HOLE0111
HOLE0112
HOLE0113
HOLE0114
HOLE0115
HOLE0116
HOLE0117
HOLE0118
HOLE0119
HOLE0120
HOLE0121
HOLE0122
HOLE0123
HOLE0124
HOLE0125
HOLE0126
HOLE0127
HOLE0128
HOLE0129
HOLE0130
HOLE0131
HOLE0132
HOLE0133
HOLE0134
HOLE0135
HOLE0136
HOLE0137
HOLE0138
HOLE0139
HOLE0140

```



```

C***** SUBROUTINE QHOLEL (THK,S,PI4,HWD,RI,ELK,ELQ,R) QHOL0000
C***** C SUBROUTINE QHOLEL FORMS STIFFNESS MATRIX AND B MATRIX QHOL0001
C THIS SUBROUTINE IS USED IN FORMING THE HOLE SUBSTRUCTURE OF HOLEL QHOL0002
C***** QHOL0003
C***** QHOL0004
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY QHOL0005
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY QHOL0006
C***** QHOL0007
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS QHOL0008
C***** QHOL0009
C***** DIMENSION S(3,3),ELK(12,12),HI(10,10),B(5,3,13),G(10,12),HIG(10,12) QHOL0010
C***** P(3,10),ELQ(12) QHOL0011
C***** COMMON/TRGFNT/CS,SS,CS2,SS2,CS4,SS4 QHOL0012
C***** CALL HMTX(PI4,HWD,RI,S,HI) QHOL0013
C***** CALL GMTX(PI4,HWD,RI,G) QHOL0014
C***** DO 10 I=1,10 QHOL0015
C***** DO 10 J=1,12 QHOL0016
C***** HIG(I,J)=0.0 QHOL0017
C***** DO 10 K=1,10 QHOL0018
C***** HIG(I,J)=HIG(I,J)+HI(I,K)*G(K,J) QHOL0019
C***** DO 20 I=1,12 QHOL0020
C***** ELQ(I)=0.0 QHOL0021
C***** DO 20 J=1,12 QHOL0022
C***** ELK(I,J)=0.0 QHOL0023
C***** DO 20 K=1,10 QHOL0024
C***** ELK(I,J)=ELK(I,J)+G(K,I)*HIG(K,J)*THK QHOL0025
C***** R=RI QHOL0026
C***** THETA=PI4/2.0 QHOL0027
C***** CALL TRIG(THETA) QHOL0028
C***** DR=(HWD/CS-RI)/5.0 QHOL0029
C***** DO 40 I=1,6 QHOL0030
C***** CALL PMTRX(P,R) QHOL0031
C***** DO 30 J=1,3 QHOL0032
C***** DO 30 K=1,12 QHOL0033
C***** B(I,J,K)=0.0 QHOL0034
C***** DO 30 L=1,10 QHOL0035
C***** B(I,J,K)=B(I,J,K)+P(J,L)*HIG(L,K)

```



```
      R(I,1,13)=THETA  
      R(I,2,13)=R  
      R(I,3,13)=0.0  
      R=R*DR  
      RETURN  
      END
```

40

```
QHOL0036  
QHOL0037  
QHOL0038  
QHOL0039  
QHOL0040  
QHOL0041
```



```

SUBROUTINE HMTX(P14,HWP,RI,S,H)
C*****
C SURROUTINE HMTX GENERATES MATRICES H AND H INVERSE
C THIS SURROUTINE IS USED IN FORMING THE HOLE SUBSTRUCTURE OF HOLEFL
C*****
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY
C*****
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS
DIMENSION H(10,10),HV(55),S(3,3),P(3,10)
DIMENSION Z(5),WZ(5)
COMMON/TRGFNT/CS,SS,CS2,SS2,CS4,SS4
F(A,B,Z)=A*B+R
DATA WZ/.2369269 ,.4786287 ,.5688889
DATA 7/-.9061798 ,-.5384693 ,.0,0F0,
E.5384693 ,.9061798 /,NZ/5/
DO 10 I=1,10
DO 10 J=1,1
H(I,J)=0.0
H(J,I)=H(I,J)
DELTHT=PI*2.0
AVGHT=DELTHT
DO 20 ITHETA=1,NZ
CALL TRIG(F(DELTHT,AVGHT,Z(ITHETA)))
RC=HWP/ABS(CS)
DELR= (RC-PI)/2.0
AVGR= (RC+PI)/2.0
DO 20 IP=1,NZ
R=F(DELTHT,AVGR,Z(IP))
CALL PMTXY(P,R)
COEFF=DELR*WZ(IR)*R*W7(ITHETA)*DELTHT
DO 20 I=1,10
DO 20 J=1,10
DO 20 K=1,3
DO 20 L=1,3

```


HMTR0036
 HMTR0037
 HMTR0038
 HMTR0039
 HMTR0040
 HMTR0041
 HMTR0042
 HMTR0043
 HMTR0044
 HMTR0045
 HMTR0046
 HMTR0047
 HMTR0048
 HMTR0049
 HMTR0050

```

20  H(I,J)=H(I,J)*P(K,I)*S(K,L)*P(L,J)*COEFF
    L=0
    DO 30 J=1,10
      DO 30 I=1,J
        L=L+1
      HW(L)=H(I,I)
      CALL SIHV(HV,10,1,0E-03,INDCTR)
      L=0
      DO 40 J=1,10
        DO 40 I=1,J
          L=L+1
        H(I,J)=HV(L)
      H(J,I)=H(I,J)
40  RETURN
    END
  
```



```

20  DO 20 I=1,10
   DO 20 J=1,12
   DO 20 K=1,3
   DO 20 L=1,2
      G(I,J)=G(I,J)*P(K,I)*RMTNT(K,L)*RL(L,J)*DS
      C  BOUNDARY 2-3 & 6-1
      DO 30 I8=1,2
         THETA=THETA1(3-I8)
         NODE1=2*(2-I8)*6*(I8-1)
         NODE2=3*(2-I8)*(I8-1)
         CALL TRIG(THETA)
         RO=MWD/ABS(CS)
         DELR=(RI-RO)/2.0*(-1.0)*(I8-1)
         AVGR=(RO+RI)/2.0
         DS=ABS(DELR)
         DO 30 I9=1,N7
            R=F(DELR*AVGR*2(I9))
            CALL PMTRX(P,R)
            CALL MMTRX(NODE1,NODE2,RMTNT)
            CALL LMTRX(NODE1,NODE2,THETA,THETA,RI,RO,R,RL)
            DDS=DS*WZ(I9)
      DO 30 I=1,10
      DO 30 J=1,12
      DO 30 K=1,3
      DO 30 L=1,2
         G(I,J)=G(I,J)*P(K,I)*RMTNT(K,L)*RL(L,J)*DDS
         C  BOUNDARY 1-(1-1)
         DTHT=PI/4/3.0
         THETA1(1)=PI/4
         NODE1=2
         DO 50 ITHETA=1,3
            NODE1=NODE1+1
            NODE2=NODE1+1
            THETA1(2)=THETA1(1)-DTHT
            DELYHT=(THETA1(2)-THETA1(1))/2.0
            AVGTHT=(THETA1(2)+THETA1(1))/2.0

```

```

GMTR0036
GMTR0037
GMTR0038
GMTR0039
GMTR0040
GMTR0041
GMTR0042
GMTR0043
GMTR0044
GMTR0045
GMTR0046
GMTR0047
GMTR0048
GMTR0049
GMTR0050
GMTR0051
GMTR0052
GMTR0053
GMTR0054
GMTR0055
GMTR0056
GMTR0057
GMTR0058
GMTR0059
GMTR0060
GMTR0061
GMTR0062
GMTR0063
GMTR0064
GMTR0065
GMTR0066
GMTR0067
GMTR0068
GMTR0069
GMTR0070
GMTR0071

```


GMTR0072
GMTR0073
GMTR0074
GMTR0075
GMTR0076
GMTR0077
GMTR0078
GMTR0079
GMTR0080
GMTR0081
GMTR0082
GMTR0083
GMTR0084
GMTR0085
GMTR0086
GMTR0087

```

NS=DELTMT*J
DO 60 I=1,NZ
  THETA=F(DEL TMT,AVGTHI,7(I,I-T))
  CALL THIG(THETA)
  CALL PMTRX(P,RI)
  CALL MMTRX(MODE1,NODE2,OMINT)
  CALL LMTX(MODE1,NODE2,THETA,THETA,WT,WO,R,RI)
  NS=NS+WZ(I,T)
DO 60 I=1,10
DO 60 J=1,12
DO 60 K=1,2
DO 60 L=1,2
  G(I,J)=G(I,J)+P(K,L)*PMTRX(K,L)*WL(L,J)*UDS
  THETA(1)=THETA(2)
  RETURN
END

```



```

SUMROUTINE PMTRX(P,R)
C.....
C SUBROUTINE PMTRX GENERATES P MATRIX
C THIS SUBROUTINE IS USED IN FORMING THE MOLE SUBSTRUCTURE OF MOLF
C.....
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY
C.....
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS
C.....
C DIMENSION P(1,10),A(1,10)
C COMMON/TEMPNT/CS,ST,CS2,SS2,CS4,SS4
C DATA 1/-2.0,2.0,2.0,-6.0,6.0,-6.0,0.0,12.0,6.0,-4.0,0.0,-2.0,
W      -2.0,2.0,2.0,-6.0,6.0,-6.0,0.0,12.0,6.0,-4.0,0.0,-2.0,
*1.0,-1.0,0.0,2.0,2.0,0.0,0/
P2=999
P4=920W2
DO 10 I=1,3
P(I,1)=1.0
P(I,5)=1.0
COEFF=1.0/24
DO 20 I=1,3
P(I,2)=COEFF
P(I,6)=COEFF
DO 30 I=1,3
P(I,3)=22
P(I,7)=92
COEFF=1.0/92
DO 40 I=1,3
P(I,4)=COEFF
P(I,8)=COEFF
C LAST TWO BYTES:
DO 50 I=1,3
P(I,9)=COEFF
P(I,10)=1.0
C INTRODUCE TRIG FUNCTIONS
DO 60 J=1,6

```

```

PMTR0000
PMTR0001
PMTR0002
PMTR0003
PMTR0004
PMTR0005
PMTR0006
PMTR0007
PMTR0008
PMTR0009
PMTR0010
PMTR0011
PMTR0012
PMTR0013
PMTR0014
PMTR0015
PMTR0016
PMTR0017
PMTR0018
PMTR0019
PMTR0020
PMTR0021
PMTR0022
PMTR0023
PMTR0024
PMTR0025
PMTR0026
PMTR0027
PMTR0028
PMTR0029
PMTR0030
PMTR0031
PMTR0032
PMTR0033
PMTR0034
PMTR0035

```


DMTR0036
DMTR0037
DMTR0038
DMTR0039
DMTR0040
DMTR0041
DMTR0042
DMTR0043
DMTR0044
DMTR0045
DMTR0046
DMTR0047

```

JP4=J*4
P(3,J)=P(1,J)*SS2
P(3,JP4)=P(3,JP4)*CS2
DO 60 I=1,2
  P(1,J)=P(1,J)*CS2
  60 P(1,JP4)=P(1,JP4)*SS2
  C  INTRODUCF A MAT4IX
    DO 70 I=1,3
      DO 70 J=1,10
        70 P(1,J)=P(1,J)*A(I,J)
      RETURN
    END

```



```

SUBROUTINE TRIG(THETA)
C *****
C SUBROUTINE TRIG EVALUATES TRIGONOMETRIC FUNCTIONS
C THIS SUBROUTINE IS USED IN FORMING THE HOLE SUBSTRUCTURE OF HOLEL
C *****
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY
C *****
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS
COMMON/TRGFNT/CS,SS,CS2,SS2,CS4,SS4
CS= COS(THETA)
SS= SIN(THETA)
CS2=1.0E0-2.0E0*SS*SS
SS2=2.0E0*SS*CS
CS4=1.0E0-2.0E0*SS2*SS2
SS4=2.0E0*CS2*SS2
RETURN
END
TRIG0000
TRIG0001
TRIG0002
TRIG0003
TRIG0004
TRIG0005
TRIG0006
TRIG0007
TRIG0008
TRIG0009
TRIG0010
TRIG0011
TRIG0012
TRIG0013
TRIG0014
TRIG0015
TRIG0016
TRIG0017

```


LMTR00036
 LMTR00037
 LMTR00038
 LMTR00039
 LMTR00040
 LMTR00041
 LMTR00042
 LMTR00043
 LMTR00044
 LMTR00045
 LMTR00046
 LMTR00047

```

5  RL(1,9)= (THEIA-THETAI(2))/DELTHI
   RL(2,10)=RL(1,9)
   RL(1,11)=1.0-RL(1,9)
   RL(2,12)=RL(1,11)
   GO TO 7
6  RL(1,11)=(R0-R)/DEL R
   RL(2,12)=RL(1,11)
   RL(1,1)=1.0-RL(1,11)
   RL(2,2)=RL(1,1)
7  CONTINUE
   RETURN
   END

```



```

SUBROUTINE MNMTRY(NODE1,NODE2,RMTNT)
C*****
C SUBROUTINE MNMTRX CREATES M AND N MATRICES AND FORMS THEIR PRODUCT
C THIS SUBROUTINE IS USED IN FORMING THE HOLE SUBSTRUCTURE OF HOLEL
C*****
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY
C*****
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS
DIMENSION RMTNT(3,2),RN(2,3),RM(3,3)
COMMON/TRGFNT/CS,SS,C52,S52,C54,S54
REAL NX,NY
CSCS=CS*CS
SSSS=SS*SS
SSCS=SS*CS
RM(1,1)=CSCS
RM(2,1)=SSSS
RM(3,1)=SSCS
RM(1,2)=SSSS
RM(2,2)=CSCS
RM(3,2)=-SSCS
RM(1,3)=-2.0*SSCS
RM(2,3)=-RM(1,3)
RM(3,3)=CSCS-SSSS
C DETERMINE BOUNDARY SEGMENT
IF( NODE1 .EQ. 6 .AND. NODE2 .EQ. 1 ) GO TO 40
IF( NODE1 .EQ. 1 .AND. NODE2 .EQ. 2 ) GO TO 10
IF( NODE1 .EQ. 2 .AND. NODE2 .EQ. 3 ) GO TO 20
IF( NODE1 .GE. 3 .AND. NODE2 .LE. 6 ) GO TO 30
10 NX=1.0
NY=0.0
GO TO 50
20 NX=-SS
NY=CS
GO TO 50
30 NX=-CS

```

```

MNMTO000
MNMTO001
MNMTO002
MNMTO003
MNMTO004
MNMTO005
MNMTO006
MNMTO007
MNMTO008
MNMTO009
MNMTO010
MNMTO011
MNMTO012
MNMTO013
MNMTO014
MNMTO015
MNMTO016
MNMTO017
MNMTO018
MNMTO019
MNMTO020
MNMTO021
MNMTO022
MNMTO023
MNMTO024
MNMTO025
MNMTO026
MNMTO027
MNMTO028
MNMTO029
MNMTO030
MNMTO031
MNMTO032
MNMTO033
MNMTO034
MNMTO035

```


MNMT0036
 MNMT0037
 MNMT0038
 MNMT0039
 MNMT0040
 MNMT0041
 MNMT0042
 MNMT0043
 MNMT0044
 MNMT0045
 MNMT0046
 MNMT0047
 MNMT0048
 MNMT0049
 MNMT0050
 MNMT0051
 MNMT0052

```

    NY=-SS
    GO TO 50
    40  NX=0.0
        NY=-1.0
    50  RN(1,1)=NX
        PN(1,2)=0.0
        RN(1,3)=NY
        RN(2,1)=0.0
        RN(2,2)=NY
        RN(2,3)=NX
        DO 60 I=1,3
        DO 60 J=1,2
        RMTNT(I,J)=0.0
        DO 60 K=1,3
        RMTNT(I,J)=RMTNT(I,J)+RM(K,I)*RN(J,K)
    60  RETURN
        END
  
```


APPENDIX C

```

C TEST MAIN - CONTRACT 81739 PROBLEMS 1 & 2
C *****
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY
C *****
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS
C *****
C .....INPUT DATA FOR PROBLEM #162.....
C SET VARIABLES IN SET
C 1 WIDTH,LENGTH,THK,STIFFCT,PRESS,RI,IPOFFST
C 2 A(1),A(2),IPOS(1),IPOS(2)
C 3 L,GNUJ
C 4 KT1,KT2,KT3
C =====DEFINITIONS OF INPUT VARIABLES=====
C WIDTH = WIDTH OF PLATE; LENGTH = LENGTH OF PLATE;
C THK = THICKNESS OF PLATE; STIFFCT = STIFFNESS FACTOR;
C PRESS = FARFIELD LOADING; RI = RADIUS OF HOLE;
C IPOFFSET = OFFSET INDICATOR; A(1) = LENGTH OF CRACK ONE;
C A(2) = LENGTH OF CRACK TWO; IPOS(1) = INITIAL POSITION OF CRACK ONE;
C IPOS(2) = FINAL POSITION OF CRACK ONE; E = YOUNGS MODULUS;
C GNU = POISSONS RATIO;KT1 = PRINT CONTROL FOR FEABL OPTIONAL PRINT;
C KT2 = PRINT CONTROL FOR OPTIONAL RING PRINTING;
C KT3 = PRINT CONTROL FOR OPTIONAL FANFLD PRINTING;
C *****
C DIMENSION RFF(1000),IFF(1000),REAL(8000),INTGR(8000),RRNG(12000)
C ,IRNG(12000),S(3,3),C(3,3),IBEGIN(6),IEND(6),ISZ(2),ISZSS(3),
C ,ANGLE(2,2),A(2),IPOS(2),ISZFF(3),IBGFF(6),ISAVE(6),ELQ(18),RK(2)
C REAL LENGTH
C INTEGER CNODE,PRNTSV
C COMMON/IO/KR,K,KP,KT1,KT2,KT3
C COMMON/SIZE/NET,NDT
C COMMON/BEGIN/ICON,IKOUNT,ILNZ,IMASTR,IQ,IK
C COMMON/END/LCON,LKOUNT,LLNZ,LMASTR,LG,LK
C COMMON/SIZESS/NETSS,NDTSS,VIDSS
C COMMON/BEGSS/IRGSS(6)
C COMMON/MILPRM/SMU,ETA
C COMMON/K1/HCRK(2,2,1H)

```



```

EQUIVALENCE (RFF(1),IFF(1)),(REAL(1),INTGR(1)),(IBEGIN(1),ICON),
@ (IEND(1),LCON),(ISZ(1),NET),(ISZSS(1),NETSS),(RRNG(1),IRNG(1))
DATA NSZFF/10000/,NSZRNG/12000/,NSZMN/8000/
DATA PI/3.141593/,S/1.0,3*0.0,1.0,4*0.0/,C/1.0,3*0.0,1.0,4*0.0/
KR=5
KW=6
DTHETA=PI/12.0
SQRTPI=SQRT(PI)

C INPUT DATA:
5000 READ(KR,5000) WIDTH,LENGTH,THK,STFFCT,PRESS,RI,IOFFST
    FORMAT(6E10.3,I5)
5001 READ(KR,5001) A(1),A(2),IPOS(1),IPOS(2)
    FORMAT(2E10.3,2I5)
5002 READ(KR,5002) E,GNUJ
    FORMAT(2E10.3)
5003 READ(KR,5003) KT1,KT2,KT3
    FORMAT(3I5)
    ANGLE(1,1)=(IPOS(1)-1)*15.0
    ANGLE(1,2)=(IPOS(2)-1)*15.0
    ANGLE(2,1)=ANGLE(1,1)+180.0
    ANGLE(2,2)=ANGLE(1,2)+180.0
    WRITE(KR,6000) WIDTH,LENGTH,THK,STFFCT,PRESS,RI,IOFFST
6000 FORMAT(1H1,S8X,14HCONTRACT #1739,/,57X,18HPROBLEMS NO. 1 & 2,/,/,6
    X,11H INPUT DATA,/,/,16H PLATE GEOMETRY:,,/,13H PLATE WIDTH=,E12.
    5,/,14H PLATE LENGTH=,E12.5,/,17H PLATE THICKNESS=,E12.5,/,24H PLA
    TE STIFFENER FACTOR=,F8.3,/,25H FAR FIELD LOADING (PSI)=,E12.5,/,1
    6H RADIUS OF HOLE=,E12.5,/,23H HOLE OFFSET INDICATOR=,I3,27H(=0, H
    OLE REMAINS CENTERED),/,12H CRACK DATA:,,/)
    NCRK=0
    DO 10 I=1,2
10 IF ( A(I) .GT. 0.0) NCRK=NCRK+1
    DO 15 I=1,NCRK
15 WRITE(KR,6001) I,A(I),(IPOS(J),ANGLE(I,J),J=1,2)
6001 FORMAT(11H CRACK NO. ,I3,17H : CRACK LENGTH=,E12.5,/,18X,23HINITI
    AL CRACK POSITION=,I3,1H(,F8.3,9H DEGREES),/,20X,21HFINAL CRACK PO
    SITION=,I3,1H(,F8.3,9H DEGREES),/)

```



```

WRITE(KW*6002) E,GNU
6002 FORMAT(21H0MATERIAL PROPERTIES:,//,20H YOUNGS MODULUS (E)=,E12.5,/,
W,16H POISSONS RATIO=,E12.5,////)
C CALCULATE S&C MATRICES:
C(2,1)=GNU
C(3,3)=(1.0-GNU)/2.0
S(2,1)=-GNU
S(3,3)=(1.0+GNU)*2.0
SMU=E/(2.0*(1.0+GNU))
ETA=(3.0-GNU)/(1.0+GNU)
GNU=E/(1.0-GNU*GNU)
DO 20 I=1,3
DO 20 J=1,I
S(I,J)=S(I,J)/E
S(J,I)=S(I,J)
C(I,J)=C(I,J)*GNU
C(J,I)=C(I,J)
20 C DETERMINE NUMBER OF QUADRILATERALS NEEDED ACROSS WIDTH:
NELW=WIDTH/(4.0*RI)
C INSURE THAT NELW IS EVEN VALUED:
I=NELW/2
I=I*2
IF(NELW.NE.I) NELW=NELW-1
W=WIDTH/NELW
RATIO=W/RI
C DETERMINE STIFFENER THICKNESS:
STFTHK=THK*STFFCT*WIDTH*THK/W
C CENTER NODE REFERENCE (INPUT FOR SUBROUTINE CZONE):
CNODE=NELW/2+1
LIMIT=2
IF(STFFCT.NE.0.) LIMIT=3
C HOLE WILL INITIALLY BE PLACED IN CENTER OF PLATE. IF IOFFST IS NOT
C EQUAL TO 0 THE HOLE CENTER WILL BE MOVED TO THE RIGHT IN INCREMENTS 0
C W AS FAR AS IS POSSIBLE
C
C PRINT OUT PROGRAM STATISTICS TO DATE:

```

PAN0073
 PAN0074
 PAN0075
 PAN0076
 PAN0077
 PAN0078
 PAN0079
 PAN0080
 PAN0081
 PAN0082
 PAN0083
 PAN0084
 PAN0085
 PAN0086
 PAN0087
 PAN0088
 PAN0089
 PAN0090
 PAN0091
 PAN0092
 PAN0093
 PAN0094
 PAN0095
 PAN0096
 PAN0097
 PAN0098
 PAN0099
 PAN0100
 PAN0101
 PAN0102
 PAN0103
 PAN0104
 PAN0105
 PAN0106
 PAN0107
 PAN0108


```

        IF(KT1.EQ.KW) GO TO 30
        WRITE(KW,6003) NELW,W,RATIO,CNODE,STFTHK,SMU,ETA
6003  FORMAT(20H0PROGRAM STATISTICS:,//,33H NUMBER OF ELEMENTS ACROSS WI
        BDTM=,I4,/,27H QUADRILATERAL DIMENSION #=,E12.5,/,30H HOLE ELEMENT
        DIMENSION RATIO=,F8.3,/,23H CENTER NOUE REFERENCE=,I4,/,21H STIFFE
        WNER THICKNESS=,E12.5,/,21H MATERIAL PARAMETERS:/,5H SMU=,E12.5,/
        0.5H ETA=,E12.5,/,10H S MATRIX:,//)
        WRITE(KW,6004) ((S(I,J),J=1,3),I=1,3)
6004  FORMAT(1H,3E12.5)
        WRITE(KW,6005)
6005  FORMAT(10H0C MATRIX:)
        WRITE(KW,6004) ((C(I,J),J=1,3),I=1,3)
30    CONTINUE
        E=0.0
C    OBTAIN FAR FIELD SUBSTRUCTURE (CHESIRE CAT):
32    CONTINUE
        PRNTSV=KT1
        KT1=KT2
        RO=(1.0*DTMETA)*40RI
        INDCTR=0
        CALL FARFLD(WIDTH,LENGTH,NELW,THK,STFTHK,C,S,CNODE,RO,PRESS,INDCTR
        W,NSZFF,RFF,IFF,NSZMN,REAL,INTGR)
        JLOC=IPOS(1)
        KT1=PRNTSV
        DO 35 I=1,3
35    ISZFF(I)=ISZSS(I)
        DO 36 I=1,6
36    IBGFF(I)=IBGSS(I)
C    SET UP CRACK PROBLEM:
        NET=2
        NDT=96*2*NCRK
        IPNTR=2*NCRK*146
        CALL SETUP(NSZMN,1,IPNTR,REAL,INTGR)
C    SET UP ASSEMBLY LIST:
C    ELEMENT NO. 1 IS THE FAR FIELD SUBSTRUCTURE (CHESIRE CAT)
        IPNTR=IMASTH*2

```

PAN0109
 PAN0110
 PAN0111
 PAN0112
 PAN0113
 PAN0114
 PAN0115
 PAN0116
 PAN0117
 PAN0118
 PAN0119
 PAN0120
 PAN0121
 PAN0122
 PAN0123
 PAN0124
 PAN0125
 PAN0126
 PAN0127
 PAN0128
 PAN0129
 PAN0130
 PAN0131
 PAN0132
 PAN0133
 PAN0134
 PAN0135
 PAN0136
 PAN0137
 PAN0138
 PAN0139
 PAN0140
 PAN0141
 PAN0142
 PAN0143
 PAN0144


```

      INTGR(IMASTR)=IPNTR
      DO 40 I=1,48
      INTGR(IPNTR)=I
      IPNTR=IPNTR+1
40  C ELEMENT NO. 2 IS THE CRACKED RING
      INTGR(IMASTR+1)=IPNTR
45  IPNTR=INTGR(IMASTR+1)
      DO 50 I=+9,NDT
      INTGR(IPNTR)=I
      IPNTR=IPNTR+1
50  J=(4*JLOC)*2-1
      IF(JLOC .GE. 21) J=(JLOC-20)*2-1
      DO 60 I=J,48
      INTGR(IPNTR)=I
      IPNTR=IPNTR+1
60  J=J+1
      IF(JLOC .EQ. 21) GO TO 75
      DO 70 I=1,J
      INTGR(IPNTR)=I
      IPNTR=IPNTR+1
70  75 CONTINUE
      IF(KT1 .EQ. KW) GO TO 80
      J=IMASTR+1
      WRITE(KW,6006) (INTGR(I),I=IMASTR,J)
6006 FORMAT(22H0MASTER ASSEMBLY LIST:./,59H POINTER FOR FAR FIELD SUBST
      RUCTURED ELEMENT (CHESIRE CAT)=,15,/,26H POINTER FOR CRACKED RING=
      ,15,/)
      J=J+1
      IPNTR=J+47
      WRITE(KW,6007) (INTGR(I),I=J,IPNTR)
6007 FORMAT(44H0FAR FIELD SUBSTRUCTURED ELEMENT D.O.F.S: ,1615,/,44X,1
      ,1615,/,44X,1615)
      IPNTR=IPNTR+1
      J=IPNTR+95*NCCK*2
      WRITE(KW,6008) (INTGR(I),I=IPNTR,J)
6008 FORMAT(25H0CRACKED RING D.O.F.S: ,1615,/,25X,1615,/,2

```


PAN0181
PAN0182
PAN0183
PAN0184
PAN0185
PAN0186
PAN0187
PAN0188
PAN0189
PAN0190
PAN0191
PAN0192
PAN0193
PAN0194
PAN0195
PAN0196
PAN0197
PAN0198
PAN0199
PAN0200
PAN0201
PAN0202
PAN0203
PAN0204
PAN0205
PAN0206
PAN0207
PAN0208
PAN0209
PAN0210
PAN0211
PAN0212
PAN0213
PAN0214
PAN0215
PAN0216

```

      85X.1615,/.25X.1615,/.25X.1615,/.25X.1615)
      80 CALL QMK(NSZMN,REAL,INTGR)
      C ASSEMBLE FAK FIELD SUBSTRUCTURE:
        DO 85 I=1,3
          85 ISZS(I)=ISZFF(I)
          DO 86 I=1,6
            86 IHGS(I)=IHGFF(I)
          CALL ASMSUB(I,IFF,IFF,REAL,INTGR)
      C COMPUTE AND ASSEMBLE CRACKED RING ELEMENT:
        THICKK=(JLOC-1)*DTMETHA
        INDCTR=0
        PHNTSV=KT1
        KT1=KT3
        CALL RING(RI,THK,THICKK,NCHK,A(1),A(2),INDCTR,S,C,NSZRNG,RRNG,
          RING)
        KT1=PHNTSV
        CALL ASMSUB(2,RRNG,IRNG,REAL,INTGR)
      C OBTAIN SOLUTION:
        I=1
        CALL FACT(I,REAL,INTGR)
        CALL SIMULQ(IGNU,REAL,INTGR)
      C OBTAIN CRACKED RING INTERIOR DISPLACEMENTS:
        CALL QBACK(2,RRNG,IRNG,REAL,INTGR)
        IF(NCHK.EQ.0) GO TO 120
      C OBTAIN KI AND KII:
        WRITE(KW,6012)
        6012 FORMAT(1H0)
        DO 90 I=1,6
          ISAVE(I)=IBEGIN(I)
          90 IBEGIN(I)=IHGS(I)
          THICKK=THICKK*100.0/PI
          DO 110 I=1,NCHK
            CALL ATTRACT(I,18,18,ELU,RRNG,IRNG)
          DO 100 J=1,2
            RK(J)=0.0
          DO 100 K=1,18

```



```

100 RK(J)=RK(J)+BCRK(I,J,K)*ELU(K)
    DO 105 J=1,2
105  RK(J)=RK(J)+SQRT=I
    RK(2)=ABS(RK(2))
    WRITE(KW+6010) I,THYCRK,(RK(J),J=1,2)
6010 FORMAT(10H CRACK NO.,12,8M ANGLE=,F8.3,8M
    *E12.5)
110 THYCRK=THYCRK+180.0
C INCREMENT CRACK POSITION:
120 JLOC=JLOC+1
    DO 125 I=1,6
125  IBEGIN(I)=ISAVE(I)
    IF(JLOC.LE.IPOS(2)) GO TO *5
C INCREMENT HOLE POSITION:
    IF(IOFFST.EQ.0) GO TO 130
    IF(CNODE.LE.LIMIT) GO TO 130
    E=E*W
    CNODE=CNODE+1
    WRITE(KW+6011) E
6011 FORMAT(14I,53X,24H INCREMENT HOLE POSITION,/,54X,13HMOLE OFFSET =
    *E12.5,/)
    GO TO 32
130 CONTINUE
    STOP
    END

```

PAN0217
 PAN0218
 PAN0219
 PAN0220
 PAN0221
 PAN0222
 PAN0223
 PAN0224
 PAN0225
 PAN0226
 PAN0227
 PAN0228
 PAN0229
 PAN0230
 PAN0231
 PAN0232
 PAN0233
 PAN0234
 PAN0235
 PAN0236
 PAN0237
 PAN0238
 PAN0239
 PAN0240
 PAN0241


```

SUBROUTINE LUG(WIDTH,LENGTH,NELW,THK,STFTHK,C,NSIZE,RSS,ISS)
C.....
C SUBROUTINE LUG GENERATES A RECTANGULAR REGION WHICH IS USED BY
C SUBROUTINES FARFLD, SBL AND DBL
C.....
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY
C.....
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS
  DIMENSION PSS(1),ISS(1),C(3,3),ELK(36),ELKSTF(36)
  P,ANGLE(4),TFMP(4),COORD(12),ND(4),ELQ(8)
  INTEGER ELNO
  REAL LENGTH,L
  COMMON/IO/FP,KW,KP,KY1,KY2,KY3
  COMMON/SIZE/NET,NDT
  COMMON/REGIN/IREGIN(6)
  COMMON/END/IENTD(6)
  COMMON/STFSS/NETSS,NDTSS,NIDSS
  COMMON/REGSS/IRGSS(6)
  COMMON/STPSS/ R(3,9),BSTF(3,9)
  DATA TEMP/40.0/
  DO 5 I=1,12
    COORD(I)=0.0
    W=WIDTH/NELW
    AR=5.0
    NELL=LENGTH/(W*AR)+1
    NDW=NELW+1
    NDL=NELL+1
    NET=NELL*NELW
    NDT=(NDL+1)*NDW+2
    NIDSS=NDT-4*NDW
    IMASTR=NET+9
    L=LENGTH/NELL
    AR=L/W
    COORD(1)=W
    COORD(2)=L

```

```

LUG 0000
LUG 0001
LUG 0002
LUG 0003
LUG 0004
LUG 0005
LUG 0006
LUG 0007
LUG 0008
LUG 0009
LUG 0010
LUG 0011
LUG 0012
LUG 0013
LUG 0014
LUG 0015
LUG 0016
LUG 0017
LUG 0018
LUG 0019
LUG 0020
LUG 0021
LUG 0022
LUG 0023
LUG 0024
LUG 0025
LUG 0026
LUG 0027
LUG 0028
LUG 0029
LUG 0030
LUG 0031
LUG 0032
LUG 0033
LUG 0034
LUG 0035

```



```

COORD(5)=L
COORD(10)=W
LMASTR=NDW*2
IF( KY1.EQ. KW ) GO TO 10
WRITE(1,*)4000*WIDTH,LENGTH,THK*STFTHK,NELW
4000 FORMAT(111.40X,9HENTRY LUG.//.23H SUBROUTINE INPUT DATA:./.11H LUG LUG 0036
*G WIDTH=.E12.5./.12H LUG LENGTH=.E12.5./.15H LUG THICKNESS=.E12.5.LUG 0037
*//.25H LUG STIFFENER THICKNESS=.E12.5.30H ( LEFT SIDE STIFFENED OLUG 0038
*ONLY)./.13H NUMBER OF ELEMENTS ACROSS WIDTH=.13.// LUG 0039
WRITE(1,*)4001*NEL,NEL,NDW,NDL,NET,NDT,MIDSS,(COORD(1),I=1,12),AR LUG 0040
4001 FORMAT(27H0SUBROUTINE LUG STATISTICS:./.37H NUMBER OF ELEMENTS ALLUG 0041
*ONG LUG LENGTH=.13.//.24H NUMBER OF NODES ALONG WIDTH=.13.//.30H NUMLUG 0042
*BER OF NODES ALONG LENGTH=.13.//.23H TOTAL NUMBER ELEMENTS=.13.//.25LUG 0043
*H TOTAL NUMBER OF O.O.F.S=.//.67H NUMBER OF O.C.F.S TO RE CONDENSEDLUG 0044
*OUT GLOBAL STIFFNESS MATRIX=.13.//.21H ELEMENT COORDINATES:./12FR.3LUG 0045
*//.22H ELEMENT ASPECT RATIO=.F4.3) LUG 0046
10 CALL SETUP(NSIZE,LMASTR,IMASTR,RSS,ISS) LUG 0047
ELNO=0 LUG 0048
IMASTR=IBEGIN(4) LUG 0049
DO 10 I=1,NEL LUG 0050
INODE1=(I-1)*NDW+1 LUG 0051
LNODE1=INODE1*NELW-1 LUG 0052
DO 20 II=INODE1,LNODE1 LUG 0053
IPNTR=IMASTR+NFT*ELNO+9 LUG 0054
ISS(IMASTR*FLNO)=IPNTR LUG 0055
ELNO=ELNO+1 LUG 0056
ISS(IPNTR+1)=11*2 LUG 0057
ISS(IPNTR)=ISS(IPNTR+1)-1 LUG 0058
ISS(IPNTR+2)=ISS(IPNTR+1)+1 LUG 0059
ISS(IPNTR+3)=ISS(IPNTR+2)+1 LUG 0060
LNODE1=INODE1*NDW+NELW LUG 0061
DO 10 II=1,NELW LUG 0062
ISS(IPNTR+5)=LNODE1*2 LUG 0063
ISS(IPNTR+4)=ISS(IPNTR+5)-1 LUG 0064
ISS(IPNTR+6)=ISS(IPNTR+4)-2 LUG 0065
ISS(IPNTR+7)=ISS(IPNTR+6)+1 LUG 0066
IPNTR=IPNTR-8 LUG 0067
LUG 0068
LUG 0069
LUG 0070
LUG 0071

```



```

30  LNODE1=LNODE1-1
    IMASTR=IBEGIN(4) *NET
    LMASTR=IMASTR+NELW*8
    INODE1=HDT-2*NDW+1
    LNODE1=1
    DO 35 I=INODE1,NDT
    DO 34 II=IMASTR,LMASTR
    IF( ISS(II) .EQ. LNODE1 ) ISS(II)=I
    CONTINUE
34  LNODE1=LNODE1+1
35  IF(KT1 .EQ. KW ) GO TO 60
36  WRITE(KW,6002)
6002 FORMAT(22HMASTER ASSEMBLY LIST,/,18H ELEMENT POINTERS:,/)
    IMASTR=IBEGIN(4)
    LMASTR=IMASTR+NET-1
    ELNO=0
    DO 40 I=IMASTR,LMASTR
    ELNO=ELNO+1
40  WRITE(KW,6003)ELNO,ISS(I)
6003 FORMAT(13H ELEMENT NO.,15,13H POINTLR=,15)
    IMASTR=LMASTR+1
    WRITE(KW,6004)
6004 FORMAT(17H0ELEMENT D.O.F.S:,,)
    DO 50 I=1,NET
    LMASTR=IMASTR+7
    WRITE(KW,6005)I,(ISS(J),J=IMASTR,LMASTR)
50  FORMAT(12H ELEMENT NO.,13,13H D.O.F.S:,815)
    IMASTR=IMASTR+8
60  CALL QPK(NSIZE,RSS,ISS)
    CALL QUAD4(COORD,THK,TEMP,C,0,ND,ANGLE,ELQ,ELK,B,1,KW)
    AR=STFTHK/THK
    DO 70 I=1,36
70  ELKSTF(I)=ELK(I)*AR
    DO 80 I=1,3
    DO 80 J=1,9
80  BSTF(I,J)=B(I,J)*AR

```

```

LUG 0072
LUG 0073
LUG 0074
LUG 0075
LUG 0076
LUG 0077
LUG 0078
LUG 0079
LUG 0080
LUG 0081
LUG 0082
LUG 0083
LUG 0084
LUG 0085
LUG 0086
LUG 0087
LUG 0088
LUG 0089
LUG 0090
LUG 0091
LUG 0092
LUG 0093
LUG 0094
LUG 0095
LUG 0096
LUG 0097
LUG 0098
LUG 0099
LUG 0100
LUG 0101
LUG 0102
LUG 0103
LUG 0104
LUG 0105
LUG 0106
LUG 0107

```



```

NELWM1=NELW-1
ELNO=0
DO 100 IROW=1,NELL
  ELNO=ELNO+1
  CALL ASMLTV(ELNO,8,ELKSTF,ELQ,RSS,ISS)
  IF(NELWM1.LT. 2) GO TO 92
  DO 90 IHERE=2,NELWM1
    ELNO=ELNO+1
    CALL ASMLTV(ELNO,8,ELK,ELQ,RSS,ISS)
  90 ELNO=ELNO+1
  100 CALL ASMLTV(ELNO,8,ELKSTF,ELQ,RSS,ISS)
    LMASTR=NDW*2
    IMASTR=IBEGIN(5)-1
    DO 110 I=1,LMASTR
      ISS(I)=I
    110 RSS(IMASTR+I)=0.0
    CALL BCON(RSS,ISS)
  115 I=1
    CALL STACON(I,NIDSS,RSS,ISS)
    DO 120 I=1,6
      IBGSS(I)=IBEGIN(I)
    120 NETSS=NET
    NDTSS=NDT
    IF( KTI .EQ. KW) GO TO 130
    WRITE(KW,6006)
  6006 FORMAT(1H0,60X,8HEXIT LUG,/)
  130 CONTINUE
    RETURN
  END

```

```

LUG 0108
LUG 0109
LUG 0110
LUG 0111
LUG 0112
LUG 0113
LUG 0114
LUG 0115
LUG 0116
LUG 0117
LUG 0118
LUG 0119
LUG 0120
LUG 0121
LUG 0122
LUG 0123
LUG 0124
LUG 0125
LUG 0126
LUG 0127
LUG 0128
LUG 0129
LUG 0130
LUG 0131
LUG 0132
LUG 0133
LUG 0134
LUG 0135
LUG 0136

```



```

SUBROUTINE CZONE(WIDTH,NELW,THK,STFTHK,C,S,CNODE,RI,NSIZE,RCZONE, CZN 0000
@ICZONE,RHOLE,IHOLE) CZN 0001
C***** CZN 0002
C SUBROUTINE CZONE GENERATES THE CENTRAL STRIP THAT CONTAINS THE HOLE CZN 0003
C FOR USE IN THE FARFIELD MESH GENERATED BY SUBROUTINE FARFLD CZN 0004
C***** CZN 0005
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY CZN 0006
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY CZN 0007
C***** CZN 0008
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS CZN 0009
DIMENSION RCZONE(1),ICZONE(1),RHOLE(1),IHOLE(1),C(3,3),S(3,3),ELK(CZN 0010
@36),ELKSTF(36),ANGLE(4),TEMP(4),ND(4),COORD(12),ELO(8),NOTASM(4), CZN 0011
@IELSTF(4),IDOF(8),RHOLE(6,3,13) CZN 0012
INTEGER CNODE,ELNO CZN 0013
COMMON/IO/KR,KW,KP,KT1,KT2,KT3 CZN 0014
COMMON/SIZE/NET,NDT CZN 0015
COMMON/BEGIN/IREGIN(6) CZN 0016
COMMON/END/IEND(6) CZN 0017
COMMON/SIZESS/ISZ(3) CZN 0018
COMMON/BEGSS/IRGSS(6) CZN 0019
COMMON/STRSS/B(3,9),BSTF(3,9) CZN 0020
DATA TEMP/4*0.0/ CZN 0021
DO 5 I=1,12 CZN 0022
COORD(I)=0.0 CZN 0023
W=WIDTH/NELW CZN 0024
NDW=NELW+1 CZN 0025
IF( CNODE .GT. 1 .AND. CNODE .LT. NDW ) GO TO 4 CZN 0026
WRITE(KW,6008) CNODE,NDW CZN 0027
6008 FORMAT(60H0***** PROGRAM INTERRUPT INITIATED BY SUBROUTINE CZONE **CZN 0028
@***,///,34H CENTER NODE REFERENCE PARAMETER =,I4,35H IS OUT OF RACZN 0029
@NGE OF E WIDTH (NELW),/,44H CNODE MUST BE GREATER THAN 1 AND LESSCZN 0030
@ THAN ,I4,/,53H ***** EXECUTION TERMINATED IN SUBROUTINE CZONE **CZN 0031
@***) CZN 031A
STOP CZN 0032
NET=1+2*NELW CZN 0033
NDT=2*(3*NDW+24) CZN 0034
NIDSS=NDT-4*NDW-48 CZN 0035

```



```

NOTASM(1)=CNODE-1
NOTASM(2)=CNODE
NOTASM(3)=CNODE-1+NELW
NOTASM(4)=CNODE+NELW
IELSTF(1)=1
IELSTF(2)=NELW+1
IELSTF(3)=NELW
IELSTF(4)=2*NELW
COORD(1)=W
COORD(2)=W
COORD(5)=W
COORD(10)=W
LMASTR=(NET-1)*9+65
IF(KT1.EQ.KW) GO TO 10
WRITE(KW,6000) WIDTH,NELW,THK,STFTHK,CNODE,RI
6000 FORMAT(1H1,59X,11HENTRY CZONE,/,/,23H SUBROUTINE INPUT DATA:,,/,13CZN 0051
@H PLATE WIDTH=.E12.5,/,33H NUMBER OF ELEMENTS ACROSS WIDTH=.I4,/,1CZN 0052
@7H PLATE THICKNESS=.E12.5,/,21H STIFFENER THICKNESS=.F12.5,/,28H HCZN 0053
@OLE CENTER NODE REFERENCE=.I3,/,27H HOLE ELEMENT INPUT RADIUS=.E12CZN 0054
@.5)
WRITE(KW,6001) NDW,NIDSS,(NOTASM(I),I=1,4),(IELSTF(I),I=1,4),(COORDCZN 0055
@D(I),I=1,12)
6001 FORMAT(29H0SUBROUTINE C7ONE STATISTICS:,,/,30H NUMBER OF NODES ACRCZN 0057
@OSS WIDTH=.I4,/,75H NUMBER OF INTERNAL D.O.F.S TO BE CONDENSED OUTCZN 0058
@ OF GLOBAL STIFFNESS MATRIX=.I4,/,30H ELEMENTS NOT TO BE ASSEMBLED CZN 0059
@:,4I5,/,26H ELEMENTS TO BE STIFFENED:4I5,/,21H ELEMENT COORDINATE CZN 0060
@S:12F8.3)
10 CALL SETUP(NSIZE,2,LMASTR,RCZONE,ICZONE)
ELNO=0
LMASTR=IBEGIN(4)
INODE=NDW+1
DO 30 IROW=1,2
IF(IROW.EQ.2) INODE=1
LNODE=INODE+NELW-1
DO 20 I=INODE,LNODE
IPNTR=IMASTP+NET+ELNO*8
ICZONE(IMASTR+ELNO)=IPNTR
ELNO=ELNO+1
CZN 0036
CZN 0037
CZN 0038
CZN 0039
CZN 0040
CZN 0041
CZN 0042
CZN 0043
CZN 0044
CZN 0045
CZN 0046
CZN 0047
CZN 0048
CZN 0049
CZN 0050
CZN 0051
CZN 0052
CZN 0053
CZN 0054
CZN 0055
CZN 0056
CZN 0057
CZN 0058
CZN 0059
CZN 0060
CZN 0061
CZN 0062
CZN 0063
CZN 0064
CZN 0065
CZN 0066
CZN 0067
CZN 0068
CZN 0069
CZN 0070
CZN 0071

```


20	ICZONE(IPNTR+1)=I*2	CZN 0072
	ICZONE(IPNTR)=ICZONE(IPNTR+1)-1	CZN 0073
	ICZONE(IPNTR+2)=ICZONE(IPNTR+1)+1	CZN 0074
	ICZONE(IPNTR+3)=ICZONE(IPNTR+2)+1	CZN 0075
	LNODE=NDW	CZN 0076
	IF(IROW.EQ.2) LNODE=3*NDW	CZN 0077
	DO 30 I=1,NELW	CZN 0078
	ICZONE(IPNTR+5)=LNODE*2	CZN 0079
	ICZONE(IPNTR+4)=ICZONE(IPNTR+5)-1	CZN 0080
	ICZONE(IPNTR+6)=ICZONE(IPNTR+4)-2	CZN 0081
	ICZONE(IPNTR+7)=ICZONE(IPNTR+6)+1	CZN 0082
	IPNTR=IPNTR-8	CZN 0083
	LNODE=LNODE-1	CZN 0084
30	IDOF(1)=CNODE-1+2*NDW	CZN 0085
	IDOF(2)=CNODE-1	CZN 0086
	IDOF(3)=CNODE-1+NDW	CZN 0087
	IDOF(4)=CNODE+NDW	CZN 0088
	IDOF(5)=CNODE+1+NDW	CZN 0089
	IDOF(6)=CNODE+1	CZN 0090
	IDOF(7)=CNODE+1+2*NDW	CZN 0091
	IDOF(8)=CNODF+2*NDW	CZN 0092
	IPNTR=IMASTR+NET+16*NELW	CZN 0093
	ICZONE(IMASTR+NET-1)=IPNTR	CZN 0094
	INODE=1	CZN 0095
	DO 40 I=1,8	CZN 0096
	ICZONE(IPNTR+INODE)=IDOF(I)*2	CZN 0097
	ICZONE(IPNTR+INODE-1)=ICZONE(IPNTR+INODE)-1	CZN 0098
40	INODE=INODE+2	CZN 0099
	IPNTR=IPNTR+16	CZN 0100
	INODE=2*(3*NDW+1)-1	CZN 0101
	LNODE=INODE+47	CZN 0102
	DO 50 I=INODE, LNODE	CZN 0103
	ICZONE(IPNTR)=I	CZN 0104
	IPNTR=IPNTR+1	CZN 0105
50	IF(KT1.EQ.KW) GO TO 80	CZN 0106
	WRITE(KW,6002)	CZN 0107


```

6002 FORMAT(22H0MASTER ASSEMBLY LIST:,//,18H ELEMENT POINTERS:,//)
      IMASTR=IBEGIN(4)
      LMASTR=IMASTR+NET-1
      ELNO=0
      DO 60 I=IMASTR,LMASTR
      ELNO=ELNO+1
      WRITE(KW,6003) ELNO,ICZONE(I)
6003 FORMAT(13H ELEMENT NO. =,15,13H      POINTER=,15)
      IMASTR=LMASTR+1
      WRITE(KW,6004)
6004 FORMAT(17H0ELEMENT D.O.F.S:,//)
      IPNTR=NET-1
      DO 70 I=1,IPNTR
      LMASTR=IMASTR+7
      WRITE(KW,6005) I,(ICZONE(J),J=IMASTR,LMASTR)
6005 FORMAT(12H ELEMENT NO.,15,13H      D.O.F.S:,//)
      IMASTR=IMASTR+9
      LMASTR=IEND(4)
      IMASTR=LMASTR-63
      WRITE(KW,6006) (ICZONE(J),J=IMASTR,LMASTR)
      R0 FORMAT(22H0HOLE ELEMENT D.O.F.S:,//,R(1H,21X,815,//))
      CALL ORK(NSIZE,RCZONE,ICZONE)
      CALL QUAD4(COORD,THK,TEMP,C,0,ND,ANGLE,ELQ,ELK,B,1,KW)
      RATIO=STFTHK/THK
      DO 90 I=1,36
      ELKSTF(I)=FLK(I)*RATIO
      DO 100 I=1,3
      DO 100 J=1,9
      BSTF(I,J)=B(I,J)*RATIO
      IPNTR=NET-1
      DO 140 N=1,IPNTR
      DO 110 I=1,4
      IF(N.EQ. NOTASM(I)) GO TO 140
      CONTINUE
      DO 120 I=1,4
      IF(N.EQ. IELSTF(I)) GO TO 130

```

```

CZN 0108
CZN 0109
CZN 0110
CZN 0111
CZN 0112
CZN 0113
CZN 0114
CZN 0115
CZN 0116
CZN 0117
CZN 0118
CZN 0119
CZN 0120
CZN 0121
CZN 0122
CZN 0123
CZN 0124
CZN 0125
CZN 0126
CZN 0127
CZN 0128
CZN 0129
CZN 0130
CZN 0131
CZN 0132
CZN 0133
CZN 0134
CZN 0135
CZN 0136
CZN 0137
CZN 0138
CZN 0139
CZN 0140
CZN 0141
CZN 0142
CZN 0143

```


CZN 0144
CZN 0145
CZN 0146
CZN 0147
CZN 0148
CZN 0149
CZN 0150
CZN 0151
CZN 0152
CZN 0153
CZN 0154
CZN 0155
CZN 0156
CZN 0157
CZN 0158
CZN 0159
CZN 0160
CZN 0161
CZN 0162
CZN 0163
CZN 0164
CZN 0165
CZN 0166
CZN 0167
CZN 0168
CZN 0169
CZN 0170
CZN 0171
CZN 0172
CZN 0173
CZN 0174
CZN 0175
CZN 0176
CZN 0177
CZN 0178
CZN 0179

```

120  CONTINUE
    CALL ASMLTV(N,8,ELK,ELQ,RCZONE,ICZONE)
    GO TO 140
130  CALL ASMLTV(N,8,ELKSTF,ELQ,RCZONE,ICZONE)
140  CONTINUE
    DO 150 I=1,12
150  COORD(I)=0.0
    COORD(1)=2.0*W
    COORD(4)=COORD(1)
    COORD(5)=COORD(1)
    COORD(6)=COORD(1)
    IMASTR=KT1
    KT1=KW
    CALL HOLEL(COORD,THK,S,PI,RHOLE,IHOLE,BHOLE)
    KT1=IMASTR
    CALL ASMSUB(NET,RHOLE,IHOLE,RCZONE,ICZONE)
    ICZONE(1)=CNODE*2
    ICZONE(2)=ICZONE(1)-1
    IMASTR=IBEGIN(5)-1
    DO 160 I=1,2
160  J=IMASTR+ICZONE(I)
    RCZONE(J)=0.0
    CALL BCON(RCZONE,ICZONE)
    I=1
    CALL STACON(I,NIDSS,RCZONE,ICZONE)
    DO 170 I=1,6
170  IBGSS(I)=IBEGIN(I)
    ISZ(1)=NET
    ISZ(2)=NOT
    ISZ(3)=NIDSS
    IF( KT1.EQ. KW ) GO TO 180
    WRITE(KW,6007)
6007 FORMAT(1H0,59X,10HEXIT CZONE,/)
180  CONTINUE
    RETURN
    END

```



```

SUBROUTINE FARFLD(WIDTH,LENGTH,NELW,THK,STFTHK,C,S,CNODE,RI,PRESS,FFD 0000
@INDCTR,NSIZE,RSS,ISS,NS7WRK,RWORK,IWORK) FFD 0001
C***** FFD 0002
C SUBROUTINE FARFLD GENERATES A FINITE ELEMENT MODEL ( A SUBSTRUCTURE) FFD 0003
C OF A PLATE WITH ONE HOLE THAT IS CENTERED IN THE VERTICAL DIRECTION FFD 0004
C AND IS EITHER CENTERED OR OFF-CENTERED IN THE HORIZONTAL DIRECTION FFD 0005
C***** FFD 0006
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY FFD 0007
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY FFD 0008
C***** FFD 0009
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS FFD 0010
DIMENSION C(3,3),S(3,3),RSS(1),ISS(1),RWORK(1),IWORK(1),RHOLE(2097) FFD 0011
@,IHOLF(2097),ISAVF1(2),ISAVE2(6),ISAVE3(6),IBGSSC(6),ISZSSC(3), FFD 0012
@ISZSS(3),IS7(2) FFD 0013
INTEGER CNODE FFD 0014
REAL LENGTH,LNGTH FFD 0015
COMMON/IO/KP,KW,KP,KT1,KT2,KT3 FFD 0016
COMMON/SIZE/NET,NDT FFD 0017
COMMON/REGIN/IREGIN(6) FFD 0018
COMMON/END/IEND(6) FFD 0019
COMMON/SIZESS/NETSS,NDTSS,NIDSS FFD 0020
COMMON/BEGSS/IBGSS(6) FFD 0021
EQUIVALENCE (RHOLE(1),IHOLE(1)),(ISZSS(1),NETSS),(ISZ(1),NET) FFD 0022
NET=3 FFD 0023
NDW=NELW+1 FFD 0024
NDT=8*NDW+4R FFD 0025
NIDSS=NDT-4R FFD 0026
IF (INDCTR.EQ.1) NIDSS=NIDSS-4*NDW FFD 0027
LMASTR=12*NDW+51 FFD 0028
NCON= NDW+1 FFD 0029
W=WIDTH/NELW FFD 0030
LNGTH=LENGTH/2.0-W FFD 0031
E=WIDTH/2.0-(CNODE-1)*W FFD 0032
IF(KT1.EQ.KW) GO TO 10 FFD 0033
WRITE(KW,6000) WIDTH,LENGTH,NELW,THK,STFTHK,CNODE,RI,PRESS,INDCTR FFD 0034
6000 FORMAT(1H1.59X,12HENTRY FARFLD.//,22H SUBROUTINE INPUT DATA.//,13FFD 0035

```



```

      10  PLATE WIDTH=.E12.5./,14H PLATE LENGTH=.E12.5./,33H NUMBR OF ELEFFD 0036
      20  MENTS ACROSS WIDTH=.15./,17H PLATE THICKNESS=.E12.5./,21H STIFFENEFD 0037
      30  GP THICKNESS=.E12.5./,33H HOLE CENTER NODE REFERENCE CODE=.15./,27HFFD 0038
      40  * HOLE ELEMENT INPUT RADIUS=.E12.5./,26H APPLIED TENSION PRESSURE=.FFD 0039
      50  *E12.5./,31H STATIC CONDENSATION INDICATOR=.14,37H (INDICATOR=0:FFD 0040
      60  *RESHIRE CAT PROBLEM:./,39x,73H INDICATOR=1:PLATE WITH END NODES ANFFD 0041
      70  *D WITH HOLE-NO LOADS OR B.C. APPLIED)) FFD 0042
      80  WRITE(KW,501) LGTH,F,NINSS FFD 0043
      90  4001 FORMAT(23HNSURROUTINE STATISTICS:./,12H LUG LENGTH=.E12.5./,13MH0FFD 0044
      00  *SLE OFFSET=.E12.5./,75H NUMBER OF INTERNAL D.O.F.S TO BE CONDENSED FFD 0045
      10  *ROUT OF GLOBAL STIFFNESS MATRIX=.15,/) FFD 0046
      20  CALL SETUP(NSIZE,NCON,LMASTR,PSS,ISS) FFD 0047
      30  IMASTR=IBEGIN(4) FFD 0048
      40  IDOF1=1 FFD 0049
      50  IDOF2=2*(3*NDW+1)-1 FFD 0050
      60  DO 30 N=1,2 FFD 0051
      70  IPNTR=IMASTR*NET*(N-1)*4*NDW FFD 0052
      80  ISS(IMASTR*N-1)=IPNTR FFD 0053
      90  IF( N.EQ. 2) IDOF1=(2*NDW+1)*2-1 FFD 0054
      00  IF( N.EQ. 2) IDOF2=(NDW+1)*2-1 FFD 0055
      10  LMASTR=IDOF1+2*NDW-1 FFD 0056
      20  DO 20 I=IDOF1,LMASTR FFD 0057
      30  ISS(IPNTR)=I FFD 0058
      40  IPNTR=IPNTR+1 FFD 0059
      50  LMASTR=IDOF2+2*NDW-1 FFD 0060
      60  DO 30 I=IDOF2,LMASTR FFD 0061
      70  ISS(IPNTR)=I FFD 0062
      80  IPNTR=IPNTR+1 FFD 0063
      90  ISS(IMASTR*2)=IPNTR FFD 0064
      00  IDOF1=1 FFD 0065
      10  IDOF2=(NDW+1)*2-1 FFD 0066
      20  LMASTR=NDW*2 FFD 0067
      30  DO 40 I=IDOF1,LMASTR FFD 0068
      40  ISS(IPNTR)=I FFD 0069
      50  IPNTR=IPNTR+1 FFD 0070
      60  LMASTR=4*NDW FFD 0071

```



```

50      DO 50 I=IDOF2,LMASR
      ISS(IPNTR)=I
      IPNTR=IPNTR+1
      IDOF1=8*NDW+1
      IDOF2=IDOF1+47
      DO 60 I=IDOF1,IDOF2
      ISS(IPNTR)=I
      IPNTR=IPNTR+1
      IF(KT1.EQ.KW) GO TO 44
      IDOF1=IBEGIN(4)
      IDOF2=IDOF1+2
      WRITE(KW,6002) (ISS(I),I=IDOF1,IDOF2)
      FORMAT(22HMASTER ASSEMBLY LIST:/.10H POINTERS:.3I5)
      DO 70 N=1,2
      IDOF1=IBEGIN(4)*NET*(N-1)+4*NDW
      IDOF2=IDOF1+4*NDW-1
      WRITE(KW,6003) N
      FORMAT(13H0LUG ELEMENT .12.9H D.O.F.S:./)
      DO 70 N=1,2
      IDOF1=IBEGIN(4)*NET*(N-1)+4*NDW
      IDOF2=IDOF1+4*NDW-1
      WRITE(KW,6004) (ISS(I),I=IDOF1,IDOF2)
      FORMAT(1H .20I6)
      IDOF1=IDOF2+1
      IDOF2=IEND(4)
      WRITE(KW,6005)
      FORMAT(23H0CZONE ELEMENT D.O.F.S:./)
      DO 80 I=1,2
      IDOF1=IBEGIN(4)*NET*(I-1)+4*NDW
      IDOF2=IDOF1+4*NDW-1
      WRITE(KW,6006) (ISS(I),I=IDOF1,IDOF2)
      CALL ORK(NSIZE,RSS,ISS)
      NIDSV=NTDSS
      DO 90 I=1,2
      ISAVE1(I)=ISZ(I)
      DO 100 I=1,6
      ISAVE2(I)=IBEGIN(I)
      ISAVE3(I)=IEND(I)
      CALL LUG(WIDTH,LENGTH,NELW,THK,STFTHK,C,NSZWRK,RWORK,IWORK)
      DO 110 I=1,2
      ISZ(I)=ISAVE1(I)
      DO 120 I=1,6
      ISAVE2(I)=ISAVE3(I)

```



```

120 IREGIN(I)=ISAVE2(I)
120 IEND(I)=ISAVE3(I)
130 DO 130 N=1.2
130 CALL ASMSUR(N,RWORK,IWORK,RSS,ISS)
140 ISAVE1(I)=IS7(I)
140 DO 150 I=1.6
140 ISAVE2(I)=IREGIN(I)
140 ISAVE3(I)=IEND(I)
150 CALL CZONE(WIDTH,NELW,THK,STFTHK,C.5,CNODE,RJ,NSZWRK,RWORK,IWORK,
      RMHOLE,IMOLF)
160 DO 160 I=1.2
160 IS7(I)=ISAVE1(I)
170 DO 170 I=1.4
170 IREGIN(I)=ISAVE2(I)
170 IEND(I)=ISAVE3(I)
170 IREGSS(I)=IRGSS(I)
180 DO 180 I=1.3
180 ISZSS(I)=IS7(I)
180 CALL ASMSUR(3,RWORK,IWORK,RSS,ISS)
180 NIOSS=NIOSS
IF(IMOCTR.EQ.0) GO TO 190
C PLATE WITH END NODES AND HOLE-NO R.C. OR APPLIED LOADS
1=1
CALL STACON(I,NIOSS,RSS,ISS)
GO TO 220
C CHECKS CAT PROGRAM
C APPLY R.C.
190 IOOF1=(2*NDW*1)*2
190 IOOF2=6*NDW
190 IMASTR=1
190 LMASTR=IREGIN(5)-1
200 DO 200 I=IOOF1,IOOF2*2
200 ISS(IMASTR)=I
200 RSS(IMASTR*1)=0.0
200 IMASTR=IMASTR+1

```

```

FFD 0108
FFD 0109
FFD 0110
FFD 0111
FFD 0112
FFD 0113
FFD 0114
FFD 0115
FFD 0116
FFD 0117
FFD 0118
FFD 0119
FFD 0120
FFD 0121
FFD 0122
FFD 0123
FFD 0124
FFD 0125
FFD 0126
FFD 0127
FFD 0128
FFD 0129
FFD 0130
FFD 0131
FFD 0132
FFD 0133
FFD 0134
FFD 0135
FFD 0136
FFD 0137
FFD 0138
FFD 0139
FFD 0140
FFD 0141
FFD 0142
FFD 0143

```


FFD 0144
FFD 0145
FFD 0146
FFD 0147
FFD 0148
FFD 0149
FFD 0150
FFD 0151
FFD 0152
FFD 0153
FFD 0154
FFD 0155
FFD 0156
FFD 0157
FFD 0158
FFD 0159
FFD 0160
FFD 0161
FFD 0162
FFD 0163
FFD 0164
FFD 0165
FFD 0166
FFD 0167
FFD 0168
FFD 0169
FFD 0170
FFD 0171

```

100F1=(2*NDW*1*NDW/2)*2-1
ISS(LMASTER)=100F1
RSS(LMASTER*100F1)=0.0
100F1=6*NDW*2
100F2=8*NDW*2
F= WOPRESS*TMK/2.0
DO 210 I=100F1,100F2.2
  RSS(LMASTER*I)=RSS(LMASTER*I)*F
  RSS(LMASTER*I*2)=RSS(LMASTER*I*2)*F
  F=(5*TMK-TMK)*WOPRESS/2.0
  RSS(LMASTER*100F1)=RSS(LMASTER*100F1)*F
  RSS(LMASTER*100F1*2)=RSS(LMASTER*100F1*2)*F
  RSS(LMASTER*100F2*2)=RSS(LMASTER*100F2*2)*F
  RSS(LMASTER*100F2)=RSS(LMASTER*100F2)*F
  CALL BCON(RSS,ISS)
  I=1
  CALL STACON(I,NI0SS,RSS,ISS)
220 DO 221 I=1.2
221 157SS(I)=157(I)
  152SS(3)=HI0SS
  DO 222 I=1.6
222 186SS(I)=186GIN(I)
  IF(K11.EQ.KW) GO TO 230
  WRITE(KW,6006)
6006 FORMAT(1H9.5H,11MEXIT FARFLD.//)
230 CONTINUE
  RETURN
  END

```


ARC40036
ARC40037
ARC40038
ARC40039
ARC40040
ARC40041
ARC40042
ARC40043
ARC40044
ARC40045
ARC40046
ARC40047
ARC40048
ARC40049
ARC40050
ARC40051
ARC40052
ARC40053
ARC40054
ARC40055
ARC40056
ARC40057
ARC40058
ARC40059
ARC40060
ARC40061
ARC40062
ARC40063
ARC40064
ARC40065
ARC40066
ARC40067
ARC40068
ARC40069
ARC40070
ARC40071

```

DO 40 N=1,10
NM1=N-1
IPNTR=IMASTR+ISZ(1)*NM1+6
IDMMY=IMASTR+NM1
ISS(IDMMY)=IPNTR
IF(N.NE.1) GO TO 20
NODE(3)=27
NODE(4)=28
GO TO 40
20 IF(N.NE.10) GO TO 30
NODE(3)=38
NODE(4)=37
GO TO 40
30 NODE(3)=NODE(2)-3
NODE(4)=NODE(1)+1
CONTINUE
40 DO 50 I=1,4
IADD=IPNTR+I*2-1
IDOF=NODE(1)+2
ISS(IADD-1)=IDOF-1
ISS(IADD)=IDOF
NODE(1)=NODE(4)
NODE(2)=NODE(3)
NODE(3)=54
NODE(4)=53
DO 110 N=1,20
IPNTR=IPNTR+8
IDMMY=IMASTR+N-1
ISS(IDMMY)=IPNTR
IF(N.NE.11) GO TO 70
NODE(3)=26
NODE(4)=27
GO TO 90
70 IF(N.NE.20) GO TO 80
NODE(3)=39
NODE(4)=38

```



```

      GO TO 90
      NODE(3)=NODE(2)-3
      NODE(4)=NODE(1)-3
      CONTINUE
      DO 100 I=1,4
      IADD=IPNTR+I*2-1
      IDOF=NODE(I)*2
      ISS(IADD-1)=IDOF-1
      ISS(IADD)=IDOF
      NODE(1)=NODE(4)
      NODE(2)=NODE(3)
      NODE(1)=53
      NODE(2)=52
      DO 160 N=21,30
      IPNTR=IPNTR+8
      IOMMY=IMASTR+N-1
      ISS(IOMMY)=IPNTR
      IF( N .NE. 21 ) GO TO 120
      NODE(3)=25
      NODE(4)=26
      GO TO 140
      IF( N .NE. 30 ) GO TO 130
      NODE(3)=40
      NODE(4)=39
      GO TO 140
      NODE(3)=NODE(2)-3
      NODE(4)=NODE(1)-3
      CONTINUE
      DO 150 I=1,4
      IADD=IPNTR+I*2-1
      IDOF=NODE(I)*2
      ISS(IADD-1)=IDOF-1
      ISS(IADD)=IDOF
      NODE(1)=NODE(4)
      NODE(2)=NODE(3)
      NODE(1)=52

```

80
 90
 100
 110
 120
 130
 140
 150
 160

ARC40072
 ARC40073
 ARC40074
 ARC40075
 ARC40076
 ARC40077
 ARC40078
 ARC40079
 ARC40080
 ARC40081
 ARC40082
 ARC40083
 ARC40084
 ARC40085
 ARC40086
 ARC40087
 ARC40088
 ARC40089
 ARC40090
 ARC40091
 ARC40092
 ARC40093
 ARC40094
 ARC40095
 ARC40096
 ARC40097
 ARC40098
 ARC40099
 ARC40100
 ARC40101
 ARC40102
 ARC40103
 ARC40104
 ARC40105
 ARC40106
 ARC40107


```

NODE(2)=51
DO 169 N=31,40
  IPNTR=IPNTR+8
  IDMMY=IMASTR+N-1
  ISS(IDMMY)=IPNTR
  IF( N.NE. 31 ) GO TO 161
  NODE(3)=50
  NODE(4)=25
  GO TO 162
161 IF( N.NE. 40 ) GO TO 1615
  NODE(3)=41
  NODE(4)=40
  GO TO 162
1615 NODE(3)=NODE(2)-1
  NODE(4)=NODE(1)-3
162 CONTINUE
DO 163 I=1,4
  IADD=IPNTR+I*2-1
  IDOF=NODE(I)*2
  ISS(IADD-1)=IDOF-1
163 ISS(IADD)=IDOF
  NODE(1)=NODE(4)
169 NODE(2)=NODE(3)
  CALL OPK(NSIZF,RSS,ISS)
  C OBTAIN ARC ELEMENTS:
  RI=RINFR
  RO=RI+RI*DTHT
  CS=COS(DTHT)
  SS=SIN(DTHT)
  C INNER RING ELEMENT:
  COORD4(1)=RI
  COORD4(4)=RO
  COORD4(7)=RO*CS
  COORD4(8)=RO*SS
  COORD4(10)=RI*CS
  COORD4(11)=RI*SS

```

```

ARC40108
ARC40109
ARC40110
ARC40111
ARC40112
ARC40113
ARC40114
ARC40115
ARC40116
ARC40117
ARC40118
ARC40119
ARC40120
ARC40121
ARC40122
ARC40123
ARC40124
ARC40125
ARC40126
ARC40127
ARC40128
ARC40129
ARC40130
ARC40131
ARC40132
ARC40133
ARC40134
ARC40135
ARC40136
ARC40137
ARC40138
ARC40139
ARC40140
ARC40141
ARC40142
ARC40143

```



```

CALL QUAD4(COORD4,THK,TEMP4,C,0,ND4,ANGLE4,ELG,ELK,8,1,KW)
L=0
DO 170 I=1,8
DO 170 J=1,1
L=L+1
ELK1(I,J)=ELK(L)
170 ELK1(J,I)=ELK1(I,J)
C MID RING & OUTER RING ELEMENTS ARE OF SIMILAR GEOMETRY AND HAVE SAME
C STIFFNESS MATRICES
C ROTATE ELEMENTS AROUND APC
THETA=THETA1-DTHY
NI=0
NM1=10
NM2=20
NO=30
DO 200 I=1,10
THETA=THETA+DTHY
NI=NI+1
NM1=NM1+1
NM2=NM2+1
NO=NO+1
CALL TRANS(THETA,ELK1,ELK,8)
CALL ASMLTV(NI,8,ELK,ELQ,RSS,ISS)
CALL ASMLTV(NM1,8,ELK,ELQ,RSS,ISS)
CALL ASMLTV(NM2,8,ELK,ELQ,RSS,ISS)
CALL ASMLTV(NO,8,ELK,ELQ,RSS,ISS)
200 CONTINUE
C STATIC CONDENSATION
ISIGN=1
CALL STACON(ISIGN,NID,RSS,ISS)
C RETURN CALLING PROGRAM CONTROLS TO THEIR ORIGINAL STORAGE
ISIZE(1)=ISZ(1)
ISIZE(2)=ISZ(2)
ISIZE(3)=NID
DO 300 I=1,6
IBEGIN(I)=IRG(I)

```

```

ARC40144
ARC40145
ARC40146
ARC40147
ARC40148
ARC40149
ARC40150
ARC40151
ARC40152
ARC40153
ARC40154
ARC40155
ARC40156
ARC40157
ARC40158
ARC40159
ARC40160
ARC40161
ARC40162
ARC40163
ARC40164
ARC40165
ARC40166
ARC40167
ARC40168
ARC40169
ARC40170
ARC40171
ARC40172
ARC40173
ARC40174
ARC40175
ARC40176
ARC40177
ARC40178
ARC40179

```


ARC40180
ARC40181
ARC40182
ARC40183
ARC40184
ARC40185

IBG(I)=ISAVE2(I)
IND(I)=ISAVE3(I)
ISZ(1)=ISAVE1(1)
ISZ(2)=ISAVE1(2)
RETURN
END

300


```

TRAN0000
SUBROUTINE TRANS(THETA,ELKSQ,ELKV,IDOOF)
C*****
C SUBROUTINE TRANS TRANSFORMS QUAD4 ELEMENTS AROUND THE 150 DEGREE ARC
C MESH OF SUBROUTINE ARC4 AND ALONG THE CRACK RAYS OF SUBROUTINE RING
C*****
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY
C*****
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS
DIMENSION ELKSQ(IDOOF,IDOOF),ELKV(1),T(8,8)
DO 5 I=1,IDOOF
DO 5 J=1,IDOOF
T(I,J)=0.0
CS=COS(THETA)
SS=SIN(THETA)
T(1,1)=CS
T(1,2)=SS
T(2,2)=CS
T(2,1)=-SS
DO 10 IT=2,IDOOF,2
ITM2=IT-2
DO 10 I=1,2
DO 10 J=1,2
T(ITM2+I,ITM2+J)= T(I,J)
L=0
DO 20 I=1,IDOOF
DO 20 J=1,I
L=L+1
ELKV(L)=0.0
DO 20 K=1,IDOOF
DO 20 LL=1,IDOOF
ELKV(L)=ELKV(L)+T(K,I)*ELKSQ(K,LL)*T(LL,J)
RETURN
END
TRAN0001
TRAN0002
TRAN0003
TRAN0004
TRAN0005
TRAN0006
TRAN0007
TRAN0008
TRAN0009
TRAN0010
TRAN0011
TRAN0012
TRAN0013
TRAN0014
TRAN0015
TRAN0016
TRAN0017
TRAN0018
TRAN0019
TRAN0020
TRAN0021
TRAN0022
TRAN0023
TRAN0024
TRAN0025
TRAN0026
TRAN0027
TRAN0028
TRAN0029
TRAN0030
TRAN0031
TRAN0032
TRAN0033

```



```

DATA NDCRK2/9,3,5,37,23,36,35,17,15,7,1,3,5,21,11,17,15,13,
#60,61,1,3,19,9,15,13,59,9,19/
DATA NDRNG1/38,39,40,41,42,43,44,45,46,47,18,16,14,
#71,70,69,68,67,66,65,64,63,62,61,1,3,5,37/
DATA NDRNG2/26,27,28,29,30,31,32,33,34,35,17,15,13,59,
#58,57,56,55,54,53,52,51,50,49,2,4,6,25/
C SAVE CALLING PROGRAM FEABL CONTROLS
DO 101 I=1,6
  ISAVE2(I)=IBG(I)
101 ISAVE3(I)=IEND(I)
DO 103 I=1,2
  ISAVE1(I)=ISZ(I)
103 RCRKTP(1)=RINNER+A1
  RCRKTP(2)=RINNER+A2
  DTHI=PI/12.0
  RNODE(1)=RINNER
DO 10 I=1,4
  RNODE(I+1)=RNODE(I)*(1.0+DTHI)
10 RO=RNODE(5)
  NET=20
  NDI=144
  ISZ(1)=20
  ISZ(2)=144
  DELTA=(RNODE(5)+RNODE(4))/2.0
  RNODE(5)=RNODE(4)
  RNODE(4)=DELTA
DO 40 I=1,2
  IF(RCRKTP(I)) .GT. DELTA GO TO 200
  IF(RCRKTP(I)) .EQ. RINNER GO TO 30
DO 20 J=1,3
  IF(RCRKTP(I)) .GT. RNODE(J) .AND. RCRKTP(I) .LE. RNODE(J+1)) ICASE(RNODE(J+1)) ICASE(RNODE(J))
  #I)=J
GO TO 40
30 ICASE(I)=4
40 CONTINUE
  RNODE(4)=RNODE(5)

```

```

RING0036
RING0037
RING0038
RING0039
RING0040
RING0041
RING0042
RING0043
RING0044
RING0045
RING0046
RING0047
RING0048
RING0049
RING0050
RING0051
RING0052
RING0053
RING0054
RING0055
RING0056
RING0057
RING0058
RING0059
RING0060
RING0061
RING0062
RING0063
RING0064
RING0065
RING0066
RING0067
RING0068
RING0069
RING0070
RING0071

```



```

RNODE(5)=R0
NIDSS=96
IF(INDCTR.EQ.1) GO TO 105
NIDSS=48-NCRK*2
105 CONTINUE
IF(KT1.EQ.KW) GO TO 9331
WRITE(KW,9000)RINNER,THK,NCRK,THICKR,A1,A2,INDCTR,(RCRKTP(I),I=1,2)RING0078
)
9000 FORMAT(1H1,50X,10HENTRY RING,/,23H SUBROUTINE INPUT DATA:./,17H RRING0080
RADIUS OF HOLE =,E12.5,/,19H THICKNESS OF RING=,E12.5,/,18H NUMBER RING0081
OF CRACKS=,I3,/,32H ANGLE OF CRACK NO. 1 (RADIANS)=,E12.5,42H (RING0082
#CRACK NO. 2 IS 180 DEGREES OPPOSITE ),/,20H CRACK NO. 1 LENGTH=,E1RING0083
#2.5,22H CRACK NO. 2 LENGTH=,E12.5,/,24H CONDENSATION INDICATOR=RING0084
#,I3,99H (INDICATOR=0 : D.O.F.S AROUND HOLE REMAIN : INDICARING0085
#TOR=1 : D.O.F.S AROUND HOLE ARE REMOVED),/,23H SUBROUTINE STATISRING0086
#TICS:./,52H RADIUS DIMENSION OF CRACK TIPS: R CRACK NO. 1 =,ERING0087
#12.5,19H R CRACK NO. 2=,E12.5)
WRITE(KW,9001)RNODE(I),I=1,5)
9001 FORMAT(35H INNER RADIUS OF CONCENTRIC RINGS : ./,34X,E12.5,/,34X,E1RING0090
#2.5,/,34X,E12.5,/,34X,E12.5,/,22H OUTER RADIUS OF RING OF PING=,E12.5)
WRITE(KW,9002)(ICASE(I),I=1,2)
9002 FORMAT(26H CASE REFERENCE NUMBERS : ,2IS)
WRITE(KW,9003)NIDSS
9003 FORMAT(71H NO. OF INTERNAL D.O.F.S TO BE CONDENSED OUT OF RING STIRING0095
#FFNFSS MATRIX =,IS)
9331 CALL SETUP(NSIZE,14,296,RSS,ISS)
C ESTABLISH ASSEMBLY LIST
IMASTR=IBS(4)
LMASTR=IEND(4)
IPNTR=IMASTR+NFT
ISS(IMASTR)=IPNTR
DO 50 I=1,2#
IADCE=IPNTR+I*2-2
IDOF=NDPVG(I)*2
ISS(IADD)=IDOF-1
ISS(IADD+1)=IDOF
50

```


RYNG0108
 RYNG0109
 RYNG0110
 RYNG0111
 RYNG0112
 RYNG0113
 RYNG0114
 RYNG0115
 RYNG0116
 RYNG0117
 RYNG0118
 RYNG0119
 RYNG0120
 RYNG0121
 RYNG0122
 RYNG0123
 RYNG0124
 RYNG0125
 RYNG0126
 RYNG0127
 RYNG0128
 RYNG0129
 RYNG0130
 RYNG0131
 RYNG0132
 RYNG0133
 RYNG0134
 RYNG0135
 RYNG0136
 RYNG0137
 RYNG0138
 RYNG0139
 RYNG0140
 RYNG0141
 RYNG0142
 RYNG0143

```

IPNTR=IPNTR+56
ISS(IMASTR+1)=IPNTR
DO 60 I=1,28
  IADD=IPNTR+I*2-2
  IOOF=NDPRNG2(I)*2
  ISS(IADD)=IOOF-1
  ISS(IADD+1)=IOOF
60  DO 5 I=1,2
    N=ICASE(I)
    GO TO (1,2,3,4),N
    IF(I.EQ.2) GO TO 12
    DO 11 N=3,4
      NP4=N*4
      NOTASM(NP4)=NP4
      NOTASM(N)=N
      NODEL1(22)=10
      NODEL1(25)=10
      GO TO 5
12  DO 13 N=11,12
      NP4=N*4
      NOTASM(NP4)=NP4
      NOTASM(N)=N
      NODEL2(22)=9
      NODEL2(25)=9
      GO TO 5
13  IF(I.EQ.2) GO TO 22
      DO 21 N=4,5
        NP4=N*4
        NOTASM(NP4)=NP4
        NOTASM(N)=N
        GO TO 5
21  DO 23 N=12,13
      NP4=N*4
      NOTASM(NP4)=NP4
      NOTASM(N)=N
      GO TO 5
23
  
```



```

3  IF(I.EQ.2) GO TO 32
   DO 31 N=5,6
   NP4=N*4
   NOTASM(NP4)=NP4
31  NOTASM(N)=N
   GO TO 5
32  DO 33 N=13,14
   NP4=N*4
   NOTASM(NP4)=NP4
33  NOTASM(N)=N
   GO TO 5
4  IF(I.EQ.2) GO TO 41
   NODEL1(17)=4H
   NODEL1(18)=12
   NODEL1(21)=12
   NODEL1(22)=10
   NODEL1(25)=10
   NOTASM(14)=19
   GO TO 5
41  NODEL2(17)=3A
   NODEL2(18)=11
   NODEL2(21)=11
   NODEL2(22)=9
   NODEL2(25)=9
   NOTASM(20)=20
   CONTINUE
5  DO 70 N=3,10
   IPNTW=IMASTW*NF1+112*(N-3)*8
   ISS(IMASTW*N-1)=IPNTW
   DO 70 I=1,4
   IADD=IPNTW*(I-1)*2
   ISS(IADD)=NODEL1((N-3)*4+I)*2-1
   ISS(IADD+1)=ISS(IADD)+1
   DO 70 M=1,1H
   IPNTW=IMASTW*NF1+176*(N-11)*8
   ISS(IMASTW*N-1)=IPNTW
70

```

```

RING0144
RING0145
RING0146
RING0147
RING0148
RING0149
RING0150
RING0151
RING0152
RING0153
RING0154
RING0155
RING0156
RING0157
RING0158
RING0159
RING0160
RING0161
RING0162
RING0163
RING0164
RING0165
RING0166
RING0167
RING0168
RING0169
RING0170
RING0171
RING0172
RING0173
RING0174
RING0175
RING0176
RING0177
RING0178
RING0179

```



```

      DO 80 I=1,4
      IADD=IPNTR*(I-1)*2
      ISS(IADD)=MODEL2((N-11)*4*(I-1)*2-1
      ISS(IADD+1)=ISS(IADD)+1
      DO 100 N=19,20
      IPNTR=IMASTR*NET*240*(N-19)+14
      ISS(IMASTR*N-1)=IPNTR
      DO 100 I=1,9
      IADD=IPNTR*(I-1)*2
      IF(N.EQ.20) GO TO 90
      ISS(IADD)=NDCPK1(I,ICASF(1))*2-1
      ISS(IADD+1)=ISS(IADD)+1
      GO TO 100
90     ISS(IADD)=NDCPK2(I,ICASF(2))*2-1
      ISS(IADD+1)=ISS(IADD)+1
      100 CONTINUE
      IF(KT1.EQ.KW) GO TO 1235
      WRITE(*,9004) (NOTASM(I),I=1,20)
      FORMAT(24H ELEMENTS NOT ASSEMBLED:./,20I5)
      WRITE(*,9003) (ISS(I),I=IMASTR,LMASTR)
      FORMAT(22H MASTER ASSEMBLY LIST:./,10H POINTERS:./,32H ELEMENT
      @T D.O.F.S: ELEMENTS 1-14:./,15(140,1615,/) ,./,49H ELEMENT D.O.F.S:
      @ELEMENT D.O.F.S: ELEMENT 19-20:./,2(140,1815,/) )
      1235 CONTINUE
      CALL ORK(NSIZF,RSS,ISS)
      C ASSEMBLE ELEMENTS
      THETA=TH7CRK*PI*DTHT
      CALL ARC4(RINNER,THETA,DTHT,C,THK,ELK,RARC,ISZSS,IBEGSS,BARC)
      DO 120 N=1,2
      CALL ASMSUP(N,RARC,IARC,RSS,ISS)
      THETA=TH7CRK*DTHT
      CALL TRANS(THETA,ELK,ELKV,R)
      DO 130 N=3,6
      NP4=N*R
      IF(N.EQ.NOTASM(N)) GO TO 125
      CALL ASMLTVIN,R,FLKV,ELQ,RSS,ISS)

```

```

RING0180
RING0181
RING0182
RING0183
RING0184
RING0185
RING0186
RING0187
RING0188
RING0189
RING0190
RING0191
RING0192
RING0193
RING0194
RING0195
RING0196
RING0197
RING0198
RING0199
RING0200
RING0201
RING0202
RING0203
RING0204
RING0205
RING0206
RING0207
RING0208
RING0209
RING0210
RING0211
RING0212
RING0213
RING0214
RING0215

```



```

125 CONTINUE
   IF(NP4.EQ. NOTASM(NP4)) GO TO 130
   CALL ASMLTV(NP4,R,FLKV,FLO,RSS,ISS)
130 CONTINUE
   THETA=THTCRK
   CALL TRANS(THETA,ELK,ELKV,R)
   DO 140 N=7,10
     NP4=N+8
   IF(N.EQ. NOTASM(N)) GO TO 135
   CALL ASMLTV(N,R,FLKV,FLO,RSS,ISS)
135 CONTINUE
   IF(NP4.FQ. NOTASM(NP4)) GO TO 140
   CALL ASMLTV(NP4,R,FLKV,FLO,RSS,ISS)
140 CONTINUE
   DO 150 N=10,20
     NM18=N-18
   IF(N.FQ. NOTASM(N)) GO TO 156
   C PCRK59 COORDINATES
     CS1=COS(THETA-DTMT)
     SS1=SIN(THETA-DTMT)
     CS2=COS(THETA)
     SS2=SIN(THETA)
     CS3=COS(THETA+DTMT)
     SS3=SIN(THETA+DTMT)
     XT=RCRKTP(NM18)*CS2
     YT=RCRKTP(NM18)*SS2
     RO=RNODE(ICASE(NM18)*2)
     COORD(1)=CS2*RO
     COORD(2)=SS2*RO
     DO 150 I=1,7
       J=I*2+1
     RO=RNODE(ICASE(NM18)*3-I)
     COORD(J)=RO*CS3
     COORD(J+1)=RO*SS3
     J=(I*5)+2+1
     RO=RNODE(ICASE(NM18)*I-1)

```

```

PING0216
PING0217
PING0218
PING0219
PING0220
PING0221
PING0222
PING0223
PING0224
PING0225
PING0226
PING0227
PING0228
PING0229
PING0230
PING0231
PING0232
PING0233
PING0234
PING0235
PING0236
PING0237
PING0238
PING0239
PING0240
PING0241
PING0242
PING0243
PING0244
PING0245
PING0246
PING0247
PING0248
PING0249
PING0250
PING0251

```


RING0252
 RING0253
 RING0254
 RING0255
 RING0256
 RING0257
 RING0258
 RING0259
 RING0260
 RING0261
 RING0262
 RING0263
 RING0264
 RING0265
 RING0266
 RING0267
 RING0268
 RING0269
 RING0270
 RING0271
 RING0272
 RING0273
 RING0274
 RING0275
 RING0276
 RING0277
 RING0278
 RING0279
 RING0280
 RING0281
 RING0282
 RING0283
 RING0284
 RING0285
 RING0286
 RING0287

```

150  COORD(J)=RO*CS1
      COORD(J+1)=RO*SS1
      RO=RNODE(1,CASE(NM18))
      COORD(9)=RO*CS2
      COORD(10)=RO*SS2
      COORD(11)=COORD( 9)
      COORD(12)=COORD(10)
      CALL PCRK50(2,SMU,ETA,XT,YT, COORD,EK,BCR)
      DO 155 I=1,2
      DO 155 J=1,18
155  BCRK(NM18,I,J)=BCR(I,J)
      CALL ASMLTV(N,18,EK,EO,RSS,ISS)
      GO TO 160
156  CONTINUE
      DO 157 I=1,2
      DO 157 J=1,18
157  BCRK(NM18,I,J)=0.0
162  TMETA=TMTCRK*PI
C  DETERMINE CONSTRAINED D.O.F.'S
      ICN=0
      IDIAG=0
      DO 190 I=1,144
      IDIAG=IDIAG+1
      J=ISS(IDG(2))+IDIAG-1)+IDIAG
      IF(RSS(J).NE.0.0) GO TO 190
      ICN=ICN+1
      ISS(ICN)=IDIAG
      RSS(IDG(5)) -1+IDIAG)=0.0
190  CONTINUE
      CALL ECOM(RSS,ISS)
      I=1
      CALL STACON(I,NIDSS,RSS,ISS)
      RO=RNODE(5)
      DO 195 I=1,6
      IHGSS(I)=IHG(I)
      IRG(I)=ISAVF2(I)
  
```



```

195 IEND(I)=ISAVE3(I)
    DO 196 I=1,2
      IS255(I)=IS2(I)
196  IS2(I)=ISAVE1(I)
      IS255(3)=NIDSS
      IF(KT1.EQ.KW) GO TO 197
      WRITE(KW,1234)
1234  FORMAT(1M0.60X,9HEXIT RING)
197  RETURN
200  WRITE(KW,6000)I,RCRKT(I),RNODE(4)
6000  FORMAT(14HLENGTH OF NO.12.10W CRACK IS .E12.5.81H MEASURED FROM
      #CENTER OF HOLE AND IS GREATER THAN MAXIMUM CRACK LENGTH ALLOWED BY
      #.7.17H THIS CODING OF .E12.5.42H. EXECUTION TERMINATED IN SURROU
      #INE RING.)
      STOP
      END
RING0288
RING0289
RING0290
RING0291
RING0292
RING0293
RING0294
RING0295
RING0296
RING0297
RING0298
RING0299
RING0300
RING0301
RING0302
RING0303

```



```

C TEST MAIN - CONTRACT 81739 PROBLEM 3 (LEFT SIDE STIFFENED )
C *****
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY
C *****
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS
C *****
C .....INPUT DATA FOR PROBLEM #3.....
C SET VARIABLES IN SET
C 1 WIDTH,LENGTH,THK,STFFCT,PRESS,RI,I0FFST
C 2 A(1),A(2),IPOS(1),IPOS(2)
C 3 E,GNU
C 4 KT1,KT2,KT3
C =====DEFINITIONS OF INPUT VARIABLES=====
C WIDTH = WIDTH OF PLATE: LENGTH = LENGTH OF PLATE;
C THK = THICKNESS OF PLATE: STFFCT = STIFFNESS FACTOR;
C PRESS = FARFIELD LOADING: RI = RADIUS OF HOLE;
C I0FFSET = OFFSET INDICATOR: A(1) = LENGTH OF CRACK ONE;
C A(2) = LENGTH OF CRACK TWO: IPOS(1) = INITIAL POSITION OF CRACK ONE;
C IPOS(2) = FINAL POSITION OF CRACK ONE; E = YOUNGS MODULUS;
C GNU = POISSONS RATIO;KT1 = PRINT CONTROL FOR FEABL OPTIONAL PRINT;
C KT2 = PRINT CONTROL FOR OPTIONAL FARFLD PRINTING;
C KT3 = PRINT CONTROL FOR OPTIONAL RING PRINTING;
      DIMENSION RFF(10000),IFF(10000),REAL(8000),INTGR(12000)
      @,IRNG(12000),S(3,3),C(3,3),IREGIN(6),IEND(6),ISZ(2),ISZSS(3),
      @ANGLE(2,2),A(2),IPOS(2),ISZFF(3),IBGFF(6),ISAVE(6),ELQ(18),RK(2)
      REAL LENGTH
      INTEGER CNODE,PRNTSV
      COMMON/IO/KR,KW,KP,KT1,KT2,KT3
      COMMON/SIZE/NET,NDT
      COMMON/BEGIN/ICON,IKOUNT,ILNZ,IMASTR,IQ,IK
      COMMON/END/LCGN,LKOUNT,LLNZ,LMASTR,LQ,LK
      COMMON/SIZESS/NETSS,NDTSS,NIDSS
      COMMON/BEGSS/IRGSS(6)
      COMMON/MILPRM/SMU,ETA
      COMMON/KI/RCRK(2,2,18)
      EQUIVALENCE (RFF(1),IFF(1)),(REAL(1),INTGR(1)),(IBEGIN(1),ICON),

```



```

@ (IEND(1),LCON), (ISZ(1),NEY), (ISZSS(1),NEYSS), (RRNG(1),IRNG(1))
DATA NSZFF/10000/, NSZFRNG/12000/, NSZMN/8000/
DATA PI/3.141593/, S/1.0, 3*0.0, 1.0, 4*0.0/, C/1.0, 3*0.0, 1.0, 4*0.0/
KR=5
KW=6
DTHEA=PI/12.0
SQRTPI=SQRT(PI)

C INPUT DATA:
READ(KR,5000) WIDTH,LENGTH,THK,STFFCT,PRESS,RI,IOFFST
5000 FORMAT(GE10.3,I5)
READ(KR,5001) A(1),A(2),IPOS(1),IPOS(2)
5001 FORMAT(2E10.3,2I5)
READ(KR,5002) E,GNU
5002 FORMAT(2E10.3)
READ(KR,5003) KT1,KT2,KT3
5003 FORMAT(3I5)
ANGLE(1,1)=(IPOS(1)-1)*15.0
ANGLE(1,2)=(IPOS(2)-1)*15.0
ANGLE(2,1)=ANGLE(1,1)+180.0
ANGLE(2,2)=ANGLE(1,2)+180.0
WRITE(KW,6000) WIDTH,LENGTH,THK,STFFCT,PRESS,RI,IOFFST
6000 FORMAT(1H1.58X,14HCONTACT 81739,7/57X,13HPROBLEM NO. 3,7/7.55X,19L
@LEFT SIDE STIFFENED,7/60X,11H INPUT DATA,7/7.16H PLATE GEOMETRY:LPAN0058
@7/7.13H PLATE WIDTH=E12.5,7.14H PLATE LENGTH=E12.5,7.17H PLATE THICKNESS=E12.5,7.24H PLATE STIFFENER FACTOR=F8.3,7.25H FAR FIELD
@LOADING (PSI)=E12.5,7.16H RADIUS OF HOLE=E12.5,7.23H HOLE OFFSELPAN0061
@T INDICATOR=I3.27H(=0, HOLE REMAINS CENTERED),7.12H CRACK DATA:LPAN0063
@/)
NCRK=0
DO 10 I=1,2
10 IF (A(I).GT. 0.0) NCRK=NCRK+1
DO 15 I=1,NCRK
15 WRITE(KW,6001) I,A(I), (IPOS(J),ANGLE(I,J),J=1,2)
6001 FORMAT(11H CRACK NO. >I3.17H : CRACK LENGTH=E12.5,7.18X,23HINITIAL CRACK POSITION=I3.1H(,F8.3,9H DEGREES),7.20X,21HFINAL CRACK POLPAN0071
@SITION=I3.1H(,F8.3,9H DEGREES),7/)
LPAN0037
LPAN0038
LPAN0039
LPAN0040
LPAN0041
LPAN0042
LPAN0043
LPAN0044
LPAN0045
LPAN0046
LPAN0047
LPAN0048
LPAN0049
LPAN0050
LPAN0051
LPAN0052
LPAN0053
LPAN0054
LPAN0055
LPAN0056
LPAN0057
LPAN0058
LPAN0059
LPAN0060
LPAN0061
LPAN0062
LPAN0063
LPAN0064
LPAN0065
LPAN0066
LPAN0067
LPAN0068
LPAN0069
LPAN0070
LPAN0071
LPAN0072

```



```

WRITE(KW,6002) E,GNU
6002 FORMAT(21H0 MATERIAL PROPERTIES: ,//,20H YOUNGS MODULUS (E)=,E12.5,
      6,16H POISSONS RATIO=,F12.5,///)
C CALCULATE S&C MATRICES:
  C(2,1)=GNU
  C(3,3)=(1.0-GNU)/2.0
  S(2,1)=-GNU
  S(3,3)=(1.0+GNU)*2.0
  SMU=E/(2.0*(1.0+GNU))
  ETA=(3.0-GNU)/(1.0+GNU)
  GNU=E/(1.0-GNU*GNU)
  DO 20 I=1,3
  DO 20 J=1,I
    S(I,J)=S(I,J)/E
    S(J,I)=S(I,J)
    C(I,J)=C(I,J)*GNU
    C(J,I)=C(I,J)
20
C DETERMINE NUMBER OF QUADRILATERALS NEEDED ACROSS WIDTH:
  NELW=WIDTH/(4.0*RI)
C INSURE THAT NELW IS EVEN VALUED:
  I=NELW/2
  I=I*2
  IF(NELW.NE.I) NELW=NELW-1
  W=WIDTH/NELW
  RATIO=W/RI
C DETERMINE STIFFENER THICKNESS:
  STFTHK=THK*STFFCT*WIDTH*THK/W
C CENTER NODE REFERENCE (INPUT FOR SUBROUTINE CZONE):
  CNODE=NELW/2+1
  LIMIT=2
  IF(IOFFST.EG.1) LIMIT=NELW
  IF(STFFCT.NE.0 .AND. IOFFST.EG.1) LIMIT=LIMIT-1
C HOLE WILL INITIALLY BE PLACED IN CENTER OF PLATE. IF IOFFST IS NOT
C EQUAL TO 0 THE HOLE CENTER WILL BE MOVED TO THE RIGHT IN INCREMENTS OF
C W AS FAR AS IS POSSIBLE
C

```

LPAN0073
 LPAN0074
 LPAN0075
 LPAN0076
 LPAN0077
 LPAN0078
 LPAN0079
 LPAN0080
 LPAN0081
 LPAN0082
 LPAN0083
 LPAN0084
 LPAN0085
 LPAN0086
 LPAN0087
 LPAN0088
 LPAN0089
 LPAN0090
 LPAN0091
 LPAN0092
 LPAN0093
 LPAN0094
 LPAN0095
 LPAN0096
 LPAN0097
 LPAN0098
 LPAN0099
 LPAN0100
 LPAN0101
 LPAN0102
 LPAN0103
 LPAN0104
 LPAN0105
 LPAN0106
 LPAN0107
 LPAN0108


```

C PRINT OUT PROGRAM STATISTICS TO DATE:
  IF (KT1.EQ.KW) GO TO 30
  WRITE(KW,6003) NELW,W,RATIO,CNODE,STFTHK,SMU,ETA
6003 FORMAT(20H0PROGRAM STATISTICS:,,/,,33H NUMBER OF ELEMENTS ACROSS WILPAN0109
LPAN0110
LPAN0111
LPAN0112
LPAN0113
LPAN0114
LPAN0115
LPAN0116
LPAN0117
LPAN0118
LPAN0119
LPAN0120
LPAN0121
LPAN0122
LPAN0123
LPAN0124
LPAN0125
LPAN0126
LPAN0127
LPAN0128
LPAN0129
LPAN0130
LPAN0131
LPAN0132
LPAN0133
LPAN0134
LPAN0135
LPAN0136
LPAN0137
LPAN0138
LPAN0139
LPAN0140
LPAN0141
LPAN0142
LPAN0143
LPAN0144
  @DTH=,I4,/,27H QUADRILATERAL DIMENSION W=.E12.5,/,30H HOLE ELEMENT
  @DIMENSION RATIO=.F8.3,/,23H CENTER NODE REFERENCE=,I4,/,21H STIFFEL
  @NER THICKNESS=.E12.5,/,21H MATERIAL PARAMETERS:,,/,,5H SMU=.E12.5,/,
  @.5H ETA=.E12.5,/,10H S MATRIX:,,/
  WRITE(KW,6004) ((S(I,J),J=1,3),I=1,3)
6004 FORMAT(1H,3E12.5)
  WRITE(KW,6005)
6005 FORMAT(10H0C MATRIX:)
  WRITE(KW,6004) ((C(I,J),J=1,3),I=1,3)
30 CONTINUE
  E=0.0
C OBTAIN FAR FIELD SUBSTRUCTURE (CHESIRE CAT):
32 CONTINUE
  PRNTSV=KT1
  KT1=KT2
  RO=(1.0+DTHETA)**4*RI
  INDCTR=0
  CALL FARFLD(WIDTH,LENGTH,NELW,THK,STFTHK,C,S,CNODE,RO,PRESS,INDCTR,
  @,NSZFF,RFF,IFF,NSZMN,RFAL,INTGR)
  JLOC=IPOS(1)
  KT1=PRNTSV
  DO 35 I=1,3
35 ISZFF(I)=ISZSS(I)
  DO 36 I=1,6
36 IBGFF(I)=IBGSS(I)
C SET UP CRACK PROBLEM:
  NET=2
  NDT=96+2*NCRK
  IPNTR=2*NCRK+146
  CALL SETUP(NSZMN,1,IPNTR,REAL,INTGR)
C SET UP ASSEMBLY LIST:
C ELEMENT NO. 1 IS THE FAR FIELD SUBSTRUCTURE (CHESIRE CAT)

```



```

IPNTR=IMASTR*2
INTGR(IMASTR)=IPNTR
DO 40 I=1,48
  INTGR(IPNTR)=I
  IPNTR=IPNTR+1
40 ELEMENT NO. 2 IS THE CRACKED RING
  INTGR(IMASTR*1)=IPNTR
  IPNTR=INTGR(IMASTR*1)
  DO 50 I=49,NDT
    INTGR(IPNTR)=I
    IPNTR=IPNTR+1
  J=(4*JLOC)*2-1
  IF (JLOC .GE. 21) J=(JLOC-20)*2-1
  DO 60 I=J,48
    INTGR(IPNTR)=I
    IPNTR=IPNTR+1
  J=J-1
  IF (JLOC .EQ. 21) GO TO 75
  DO 70 I=1,J
    INTGR(IPNTR)=I
    IPNTR=IPNTR+1
  70 IPNTR=IPNTR+1
  75 CONTINUE
  IF (KT1 .EQ. KW) GO TO 80
  J=IMASTR+1
  WRITE(KW,6006) (INTGR(I),I=IMASTR,J)
  6006 FORMAT(22H0MASTER ASSEMBLY LIST: /,59H POINTER FOR FAR FIELD SUBSTRUCTURED ELEMENT (CHESIRE CAT)=,15,/,26H POINTER FOR CRACKED RING=,15,/)
  J=J+1
  IPNTR=J+47
  WRITE(KW,6007) (INTGR(I),I=J,IPNTR)
  6007 FORMAT(44H0FAR FIELD SUBSTRUCTURED ELEMENT D.O.F.S: ,1615,/,44X,1)
  615,/,44X,1615)
  IPNTR=IPNTR+1
  J=IPNTR+95+NCRK*2
  WRITE(KW,6008) (INTGR(I),I=IPNTR,J)

```

LPAN0145
 LPAN0146
 LPAN0147
 LPAN0148
 LPAN0149
 LPAN0150
 LPAN0151
 LPAN0152
 LPAN0153
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 LPAN0174
 LPAN0175
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 LPAN0177
 LPAN0178
 LPAN0179
 LPAN0180


```

100 DO 100 K=1,18
    RK(J)=RK(J)+BCRK(I,J,K)*ELQ(K)
105 DO 105 J=1,2
    RK(J)=RK(J)*SQRTPI
    RK(2)=ABS(RK(2))
    WRITE(KW,6010) I,TWTCRK,(RK(J),J=1,2)
6010 FORMAT(10H CRACK NO.,12,8H ANGLE=,F8.3,8H
    KI=,E12.5,8H
    #=,E12.5)
110 TWTCRK=TWTCRK+180.0
    C INCREMENT CRACK POSITION:
120 JLUC=JLOC+1
    DO 125 I=1,6
125 IBEGIN(I)=ISAVE(I)
    IF(JLOC.LE.IPOS(2)) GO TO 45
    C INCREMENT HOLE POSITION:
    IF(IOFFST.EQ.0) GO TO 130
    IF(CNODE.GE.LIMIT.AND.IOFFST.EQ.1) GO TO 130
    IF(CNODE.LE.LIMIT.AND.IOFFST.EQ.-1) GO TO 130
    C=E-IOFFST*W
    CNODE=CNODE+IOFFST
    WRITE(KW,6011) E
6011 FORMAT(1H1.53X,24H INCREMENT HOLE POSITION,/,54X,13HHOLE OFFSET =,E12.5,///)
    GO TO 32
130 CONTINUE
    STOP
    END

```

```

LPAN0217
LPAN0218
LPAN0219
LPAN0220
LPAN0221
LPAN0222
LPAN0223
LPAN0224
LPAN0225
LPAN0226
LPAN0227
LPAN0228
LPAN0229
LPAN0230
LPAN0231
LPAN0232
LPAN0233
LPAN0234
LPAN0235
LPAN0236
LPAN0237
LPAN0238
LPAN0239
LPAN0240
LPAN0241
LPAN0242
LPAN0243

```



```

SUBROUTINE LUG(WIDTH,LENGTH,NELW,THK,STFTHK,C,NSI7E,RSS,ISS)
C*****
C SUBROUTINE LUG GENERATES A RECTANGULAR REGION WHICH IS USED BY
C SUBROUTINES FAPFLD, SBL AND DBL
C*****
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY
C*****
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS
C LEFT SIDE STIFFENED ONLY
      DIMENSION RSS(1),ISS(1),C(3,3),ELK(36),ELKSTF(36)
      P,ANGLE(4),TEMP(4),COORD(12),ND(4),ELQ(8)
      INTEGER ELNO
      REAL LENGTH,L
      COMMON/IO/KP,KW,KP,KT1,KT2,KT3
      COMMON/SIZE/NET,NDT
      COMMON/BEGIN/IBEGIN(6)
      COMMON/END/IEND(6)
      COMMON/SIZESS/NETSS,NDTSS,NIDSS
      COMMON/BEGSS/IRGSS(6)
      COMMON/STRSS/ R(3,9),BSTF(3,9)
      DATA TEMP/4*0.0/
      DO 5 I=1,12
      COORD(I)=0.0
      W=WIDTH/NELW
      AR=5.0
      NELL=LENGTH/(W*AR)+1
      NDW=NELW+1
      NDL=NELL+1
      NET=NELL*NELW
      NDT=(NDL+1)*NDW*2
      NIDSS=NDT-4*NDW
      IMASTP=NET*9
      L=LENGTH/NELL
      AR=L/W
      COORD(1)=W

```

5


```

COORD(2)=L
COORD(5)=L
COORD(10)=W
LMASTR=NDW*2
IF( KTY .EQ. KW ) GO TO 10
WRITE(KW,6066)WIDTH,LENGTH,THK,STFTMK,NELW
4000 FORMAT(1M1,60X,9HENTRY LUG,/,23H SUBROUTINE INPUT DATA:,,11H LUGL0042
      6G WIDTH=,E12.5,/,12H LUG LENGTH=,E12.5,/,15H LUG THICKNESS=,E12.5,LUGL0043
      A/,25H LUG STIFFENER THICKNESS=,E12.5,30H ( LEFT SIDE STIFFENED LUGL0044
      ONLY),/,33H NUMBER OF ELEMENTS ACROSS WIDTH=,I3,/)
      LUGL0045
      WRITE(KW,6067)NEL,NEL,NEL,NDL,NET,NDT,NIDSS,(COORD(I),I=1,12),AR
      LUGL0046
      4001 FORMAT(27H0SUBROUTINE LUG STATISTICS:,,37H NUMBER OF ELEMENTS ALLUGL0047
      30G LUG LENGTH=,I3,/,29H NUMBER OF NODES ALONG WIDTH=,I3,/,30H NUMLUGL0048
      38ER OF NODES ALONG LENGTH=,I3,/,23H TOTAL NUMBER ELEMENTS=,I3,/,25LUGL0049
      39H TOTAL NUMBER OF D.O.F.S=,/,63H NUMBER OF D.O.F.S TO BE CONDENSEDLUGL0050
      40 OUT GLOBAL STIFFNESS MATPIX=,I3,/,21H ELEMENT COORDINATES:,.12F8.3LUGL0051
      5,/,22H ELEMENT ASPECT RATIO=,F8.3)
      LUGL0052
10 CALL SETUP(NSIZE,LMASTR,IMASTP,RSS,ISS)
      LUGL0053
      ELNO=0
      IMASTR=IREGIN(4)
      LUGL0054
      DO 30 I=1,NFLI
      LUGL0055
      INODE1=(I-1)*NDW+1
      LUGL0056
      LNODE1=INODE1+NELW-1
      LUGL0057
      DC 20 II=INODE1,LNODE1
      LUGL0058
      IPNTR=IMASTR+NET+ELNO*8
      LUGL0059
      ISS(IMASTR+ELNO)=IPNTR
      LUGL0060
      ELNO=ELNO+1
      LUGL0061
      ISS(IPNTR+1)=II*2
      LUGL0062
      ISS(IPNTR)=ISS(IPNTR+1)-1
      LUGL0063
      ISS(IPNTR+2)=ISS(IPNTR+1)+1
      LUGL0064
      ISS(IPNTR+3)=ISS(IPNTR+2)+1
      LUGL0065
      LNODE1=INODE1+NDW+NELW
      LUGL0066
      DO 30 II=1,NELW
      LUGL0067
      ISS(IPNTR+5)=LNODE1*2
      LUGL0068
      ISS(IPNTR+4)=ISS(IPNTR+5)-1
      LUGL0069
      ISS(IPNTR+6)=ISS(IPNTR+4)-2
      LUGL0070
      LUGL0071

```



```

30      ISS(IPNTR*7)=ISS(IPNTR*4)+1
        IPNTR=IPNTR-8
        LNODE1=LNODE1-1
        IMASTP=IBEGIN(4) *NET
        LMASTR=IMASTR*NELEW*8
        INODE1=NDT-2*NDW+1
        LNODE1=1
        DO 35 I=INODE1,NDT
        DO 34 II=IMASTR,LMASTR
        IF( ISS(II) .EQ. LNODE1 ) ISS(II)=I
        CONTINUE
34      LNODE1=LNODE1+1
35      IF(KT1 .EQ. KW ) GO TO 40
36      WRITE(*,*)
4002   FORMAT(22HMASTER ASSEMBLY LIST:,,18H ELEMENT POINTERS:./)
        IMASTP=IBEGIN(4)
        LMASTR=IMASTR*NET-1
        ELNO=0
        DO 40 I=IMASTP,LMASTR
        ELNO=ELNO+1
        WRITE(*,*)ELNO,ISS(I)
4003   FORMAT(13H ELEMENT NO.=:IS,13H      POINTER=:IS)
        IMASTP=LMASTR+1
        WRITE(*,*)
4004   FORMAT(17H0ELEMENT 0.0.F.S:./)
        DO 50 I=1,NET
        LMASTR=IMASTR+7
        WRITE(*,*)I,(ISS(J),J=IMASTR,LMASTR)
4005   FORMAT(12H ELEMENT NO.:I3,13H      0.0.F.S:,:8I5)
50      IMASTR=IMASTR+8
60      CALL ORK(NSIZE,RSS,ISS)
        CALL CUAD4(COORD,THK,TEMP,C,0,ND,ANGLE,ELQ,ELK,B,1,KW)
        AR=STFTHK/THK
        DO 70 I=1,36
        ELKSTF(I)=ELK(I)*AR
70      DO 80 I=1,3

```

```

LUGL0072
LUGL0073
LUGL0074
LUGL0075
LUGL0076
LUGL0077
LUGL0078
LUGL0079
LUGL0080
LUGL0081
LUGL0082
LUGL0083
LUGL0084
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LUGL0086
LUGL0087
LUGL0088
LUGL0089
LUGL0090
LUGL0091
LUGL0092
LUGL0093
LUGL0094
LUGL0095
LUGL0096
LUGL0097
LUGL0098
LUGL0099
LUGL0100
LUGL0101
LUGL0102
LUGL0103
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LUGL0105
LUGL0106
LUGL0107

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LUGL0108
LUGL0109
LUGL0110
LUGL0111
LUGL0112
LUGL0113
LUGL0114
LUGL0115
LUGL0116
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LUGL0119
LUGL0120
LUGL0121
LUGL0122
LUGL0123
LUGL0124
LUGL0125
LUGL0126
LUGL0127
LUGL0128
LUGL0129
LUGL0130
LUGL0131
LUGL0132
LUGL0133
LUGL0134
LUGL0135

```

      DO 80 J=1.9
      BSTF(I,J)=B(I,J)*AR
      NELWM1=NELW-1
      ELNO=0
      DO 100 IROW=1,NELL
      DO 90 IHERE=1,NELWM1
      ELNO=ELNO+1
      90  CALL ASMLTV(ELNO,8,ELK,ELQ,RSS,ISS)
      ELNO=ELNO+1
      100 CALL ASMLTV(ELNO,8,ELKSTF,ELQ,RSS,ISS)
      LMASTP=NDW*2
      IMASTP=IBEGIN(5)-1
      DO 110 I=1,LMASTP
      ISS(I)=1
      110 PSS(IMASTP+I)=0.0
      CALL BCON(RSS,ISS)
      115 I=1
      CALL STACON(I,NIDSS,RSS,ISS)
      DO 120 I=1.5
      120 IBGSS(I)=IBEGIN(I)
      NETSS=NET
      NOTSS=NDT
      IF( KY1 .EQ. KW) GO TO 130
      WRITE(*,6006)
      6006 FORMAT(1H0,60X,8MEXIT LUG.//)
      130 CONTINUE
      RETURN
      END

```



```

SURROUTINE CZONE(WIDTH,NELW,THK,STFWK,C,S,CNODE,R),NSIZE,RCZONE, CZNL0000
  @CZONE,RHOLE,IMOLE) CZNL0001
C..... CZNL0002
C LEFT SIDE STIFFENED CZNL0003
C SURROUTINE CZONE GENERATES THE CENTRAL STRIP THAT CONTAINS THE HOLE CZNL0004
C FOR USE IN THE FARFIELD MESH GENERATED BY SUBROUTINE FARFLD CZNL0005
C..... CZNL0006
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY CZNL0007
C AEROPLASTIC AND STRUCTURES RESEARCH LABORATORY CZNL0008
C..... CZNL0009
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS CZNL0010
  DIMENSION RCZONE(1),ICZONE(1),OHOLE(1),IHOLE(1),C(3,3),S(3,3),ELK(CZNL0011
  @36),ELKSTF(36),ANGLE(4),TEMP(4),ND(4),COORD(12),ELO(8),NOTASM(4), CZNL0012
  @IELSTF(12),IDOF(8),RHOLE(6,3,13) CZNL0013
  INTEGER CNODE,ELND CZNL0014
  COMMON/IO/KP,KW,KP,KT1,KT2,KT3 CZNL0015
  COMMON/SIZE/NET,NDT CZNL0016
  COMMON/REGIN/IREGIN(4) CZNL0017
  COMMON/END/IEND(4) CZNL0018
  COMMON/SIZFSS/IS7(3) CZNL0019
  COMMON/REGSS/IRGSS(4) CZNL0020
  COMMON/SIZSS/R(3,9),BSTF(3,9) CZNL0021
  DATA TEMP/4*0.0/ CZNL0022
  DO 5 I=1,12 CZNL0023
    COORD(I)=0.0 CZNL0024
    W=WIDTH/NELW CZNL0025
    NDW=NELW*1 CZNL0026
    IF( CNODE .GT. 1 .AND. CNODE .LT. NDW ) GO TO 4 CZNL0027
    WRITE(KW,6008) CNODE,NDW CZNL0028
4008 FORMAT(60H0..... PROGRAM INTERRUPT INITIATED BY SURROUTINE CZONE **CZNL0029
    @.....//,34H CENTER NODE REFERENCE PARAMETER =,14.35H IS OUT OF RACZNL0030
    @NGE OF E WIDTH (NELW),,7.44H CNODE MUST BE GREATER THAN 1 AND LFSSCZNL0031
    @ THAN ,14.//,53H ..... EXECUTION TERMINATED IN SURROUTINE CZONE **CZNL0032
    @....) CZNL0032A
  STOP CZNL0033
  NET=1+2*NELW CZNL0034
  NDT=2+(3*NDW+24) CZNL0035

```



```

NIDSS=NDT-4*NDW-4*H
NOTASM(1)=CNODE-1
NOTASM(2)=CNODE
NOTASM(3)=CNODE-1*NELEW
NOTASM(4)=CNODE*NELEW
IELSTF(1)=NELEW
IELSTF(2)=2*NELEW
COORD(1)=W
COORD(2)=W
COORD(5)=*W
COORD(10)=W
LMASTR=(NET-1)*9*65
IF(KY1.EQ. KW) GO TO 10
WRITE(KW,600) WIDTH*NELEW,TWK*STFTWK,CNODE,PI
6000 FORMAT(1H1,4X,11X)ENTPY CZONE,/,/,23H SUBROUTINE INPUT DATA:,,/,13CZNL0050
      67H PLATE WIDTH*,E12.5,/,33H NUMBER OF ELEMENTS ACROSS WIDTH*,E14.7,/,1CZNL0051
      67H PLATE THICKNESS*,E12.5,/,21H STIFFENER THICKNESS*,E12.5,/,22H HOLE CIRCLE CENTER NODE REFERENCE*,E13.7,27H HOLE ELEMENT INPUT RADIUS*,E12CZNL0053
      65)
WRITE(KW,600) NOM.NODS,(NOTASM(1),I=1,4),(IELSTF(1),I=1,2),(COORDCZNL0055
      601),I=1,12)
6001 FORMAT(29M)SUBROUTINE CZONE STATISTICS:,,/,30H NUMBER OF NODES ACPCZNL0057
      60SS WIDTH*,E14.7,75H NUMBER OF INTERNAL D.O.F.S TO BE CONDENSED OUTCZNL0058
      4 OF GLOBAL STIFFNESS MAT*,E14.7,30H ELEMENTS NOT TO BE ASSEMBLED CZNL0059
      8:*,E15.7,26H ELEMENTS TO BE STIFFENED*,E15.7,21H ELEMENT COORDINATECZNL0060
      8:*,E12.5,3)
10 CALL SETUP,NSIZE,2,LMASTR,PCZONE,ICZONEI
      FINO=0
      IMASTR=18E6IN(4)
      INODE=NO*1
      DO 30 IPDW=1,2
      IF(IPDW.EQ. 2) INODE=1
      LNODE=INODE*NELEW-1
      DO 20 I=INODE,INODE
      IPATH=IMASTR*NET*ELNO*H
      ICZONE(IMASTR*ELNO)=IPATH
      ELNO=FINO+1

```

CZNL0036
CZNL0037
CZNL0038
CZNL0039
CZNL0040
CZNL0041
CZNL0042
CZNL0043
CZNL0044
CZNL0045
CZNL0046
CZNL0047
CZNL0048
CZNL0049
CZNL0050
CZNL0051
CZNL0052
CZNL0053
CZNL0054
CZNL0055
CZNL0056
CZNL0057
CZNL0058
CZNL0059
CZNL0060
CZNL0061
CZNL0062
CZNL0063
CZNL0064
CZNL0065
CZNL0066
CZNL0067
CZNL0068
CZNL0069
CZNL0070
CZNL0071

CZNL0072
CZNL0073
CZNL0074
CZNL0075
CZNL0076
CZNL0077
CZNL0078
CZNL0079
CZNL0080
CZNL0081
CZNL0082
CZNL0083
CZNL0084
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CZNL0087
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CZNL0089
CZNL0090
CZNL0091
CZNL0092
CZNL0093
CZNL0094
CZNL0095
CZNL0096
CZNL0097
CZNL0098
CZNL0099
CZNL0100
CZNL0101
CZNL0102
CZNL0103
CZNL0104
CZNL0105
CZNL0106
CZNL0107

```

20 ICZONE(IPNTR*1)=1*2
   ICZONE(IPNTR)=ICZONE(IPNTR*1)-1
   ICZONE(IPNTR*2)=ICZONE(IPNTR*1)*1
   ICZONE(IPNTR*3)=ICZONE(IPNTR*2)*1
   LNODE=NDW
   IF(IPND.EQ.2) LNODE=1*NDW
   DO 30 I=1,NFLW
     ICZONE(IPNTR*5)=LNODE*2
     ICZONE(IPNTR*4)=ICZONE(IPNTR*5)-1
     ICZONE(IPNTR*6)=ICZONE(IPNTR*4)-2
     ICZONE(IPNTR*7)=ICZONE(IPNTR*6)*1
     IPNTR=IPNTR-A
     LNODE=LNODE-1
     IDOF(1)=CNODE-1*2*NDW
     IDOF(2)=CNODE-1
     IDOF(3)=CNODE-1*NDW
     IDOF(4)=CNODE*NDW
     IDOF(5)=CNODE*1*NDW
     IDOF(6)=CNODE*1
     IDOF(7)=CNODE*1*2*NDW
     IDOF(8)=CNODE*2*NDW
     IPNTR=IMASTR*NET*1*2*NFLW
     ICZONE(IMASTR*NET-1)=IPNTR
     LNODE=1
   DO 40 I=1,N
     ICZONE(IPNTR*INODE)=IDOF(I)*2
     ICZONE(IPNTR*INODE-1)=ICZONE(IPNTR*INODE)-1
     INODE=INODE*2
     IPNTR=IPNTR*16
     INODE=2*(3*NDW*1)-1
     LNODE=INODE*47
   DO 50 I=INODE*LNODE
     ICZONE(IPNTR)=I
     IPNTR=IPNTR*1
     IF(KY1.EQ.KW) GO TO 40
     WRITE(KW,6002)

```

30

40

50


```

6002 FORMAT(22HMASTER ASSEMBLY LIST:,//,18H ELEMENT POINTERS:./)
      IMASTR=IBEGIN(4)
      LMASTR=IMASTR*NET-1
      ELNO=0
      DO 60 I=IMASTR,LMASTR
        ELNO=ELNO+1
        WRITE(KW,6003) ELNO,ICZONE(I)
6003 FORMAT(13H ELEMENT NO. =,I5,13H          POINTER=,I5)
        IMASTR=LMASTR+1
        WRITE(KW,6004)
6004 FORMAT(17H0ELEMENT D.O.F.S:./)
        IPNTR=NET-1
        DO 70 I=1,IPNTR
          LMASTR=IMASTR+7
          WRITE(KW,6005) I,(ICZONE(J),J=IMASTR,LMASTR)
6005 FORMAT(12H ELEMENT NO. =,I5,13H          D.O.F.S:./)
          IMASTR=IMASTR+8
          LMASTR=IEND(4)
          IMASTR=LMASTR-63
          WRITE(KW,6006) (ICZONE(J),J=IMASTR,LMASTR)
6006 FORMAT(22H0HOLE ELEMENT D.O.F.S:./,R(1H ,21X,R15,./))
80      CALL ORK(N5IZE,RCZONE,ICZONE)
      CALL QUAD4(COORD,THK,TEMP,C,0,ND,ANGLE,ELQ,ELK,R,1,KW)
      RATIO=STFTHK/THK
      DO 90 I=1,36
6009      ELKSTF(I)=FLK(I)*RATIO
      DO 100 I=1,2
      DO 100 J=1,9
100      BSTF(I,J)=B(I,J)*RATIO
      IPNTR=NET-1
      DO 140 N=1,IPNTR
      DO 110 I=1,4
110      IF(N.EQ. NOTASM(I)) GO TO 140
      CONTINUE
      DO 120 I=1,2
      IF(N.EQ. IELSTF(I)) GO TO 130

```

```

CZNL0108
CZNL0109
CZNL0110
CZNL0111
CZNL0112
CZNL0113
CZNL0114
CZNL0115
CZNL0116
CZNL0117
CZNL0118
CZNL0119
CZNL0120
CZNL0121
CZNL0122
CZNL0123
CZNL0124
CZNL0125
CZNL0126
CZNL0127
CZNL0128
CZNL0129
CZNL0130
CZNL0131
CZNL0132
CZNL0133
CZNL0134
CZNL0135
CZNL0136
CZNL0137
CZNL0138
CZNL0139
CZNL0140
CZNL0141
CZNL0142
CZNL0143

```



```

120 CONTINUE
    CALL ASMLTV(N,R,ELK,ELQ,RCZONE,ICZONE)
    GO TO 140
130 CALL ASMLTV(N,R,ELKSTF,ELQ,RCZONE,ICZONE)
140 CONTINUE
    DO 150 I=1,12
150   COORD(I)=0.0
        COORD(1)=2.0*W
        COORD(4)=COORD(1)
        COORD(5)=COORD(1)
        COORD(R)=COORD(1)
        IMASTR=KT1
        KT1=KW
        CALL HOLEL(COORD,THK,S,RI,RHOLE,IHOLE,BHOLE)
        KT1=IMASTR
        CALL ASMSUB(NET,RHOLE,IHOLE,RCZONE,ICZONE)
        ICZONE(1)=CNODE*2
        ICZONE(2)=ICZONE(1)-1
        IMASTR=IBEGIN(5)-1
        DO 160 I=1,2
160   J=IMASTR+ICZONE(I)
        RCZONE(J)=0.0
        CALL BCON(RCZONE,ICZONE)
        I=1
        CALL STACON(I,NIDSS,RCZONE,ICZONE)
        DO 170 I=1,6
170   IRGSS(I)=IREGIN(I)
        ISZ(1)=NET
        ISZ(2)=NOT
        ISZ(3)=NIDSS
        IF (KT1.EQ.KW) GO TO 180
        WRITE(KW,6007)
6007 FORMAT(1H0,59X,':0HEXIT CZONE,/')
180 CONTINUE
    RETURN
    END

```

CZNL0144
CZNL0145
CZNL0146
CZNL0147
CZNL0148
CZNL0149
CZNL0150
CZNL0151
CZNL0152
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CZNL0154
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CZNL0165
CZNL0166
CZNL0167
CZNL0168
CZNL0169
CZNL0170
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CZNL0172
CZNL0173
CZNL0174
CZNL0175
CZNL0176
CZNL0177
CZNL0178
CZNL0179


```

SUBROUTINE FARFLD(WIDTH,LENGTH,NELW,THK,SIFTHK,C,S,CNODE,RI,PRESS,FFDL0000
BINDCTR,NSIZE,RSS,ISS,NS7WRK,RWORK,IWORK)
FFDL0001
FFDL0002
FFDL0003
FFDL0004
FFDL0005
FFDL0006
FFDL0007
FFDL0008
FFDL0009
FFDL0010
FFDL0011
FFDL0012
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FFDL0035

C LEFT SIDE STIFFENED ONLY
C SURROUTINE FARFLD GENERATES A FINITE ELEMENT MODEL ( A SUBSTRUCTURE )
C OF A PLATE WITH ONE HOLE THAT IS CFENTERED IN THE VERTICAL DIPECTION
C AND IS EITHER CFENTERED OR OFF-CENTERED IN THE HORIZONTAL DIRECTION
C *****
C COPYRIGHT (C) 1975 MASSACHUSETTS INSTITUTE OF TECHNOLOGY
C AEROELASTIC AND STRUCTURES RESEARCH LABORATORY
C *****
C FINITE ELEMENT APPLICATIONS TO USAF STRUCTURAL INTEGRITY PROBLEMS
DIMENSION C(3,3),S(3,3),RSS(1),ISS(1),RWORK(1),IWORK(1),RHOLE(2097FFDL0012
@),IHOLE(2097),ISAVE1(2),ISAVE2(6),ISAVE3(6),IRGSSC(6),ISZSSC(3),
@ISZSS(3),IS7(2)
INTEGER CNODE
REAL LENGTH,LENGTH
COMMON/IO/KP,KW,KT1,KT2,KT3
COMMON/SIZE/NET,NDY
COMMON/BEGIN/IBEGIN(6)
COMMON/END/ITEND(6)
COMMON/SIZESS/NETSS,NDTSS,NIDSS
COMMON/REGSS/IRGSS(6)
EQUIVALENCE (RHOLE(1),IHOLE(1)),(IS7SS(1),NETSS),(IS7(1),NET)
NET=3
NDW=NELW+1
NDT=8*NDW+4R
NIDSS=NDT-4R
IF (INDCTR.EQ.1) NIDSS=NIDSS-4*NDW
LMASTR=12*NDW+51
NCON= NDW+1
W=WIDTH/NELW
LNGTH=LENGTH/2.0-W
E=WIDTH/2.0-(CNODE-1)*W
IF (KT1.EQ.KW) GO TO 10
WRITE (KW,6000) WIDTH,LENGTH,NELW,THK,SIFTHK,CNODE,RI,PRESS,INDCTR

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6000 FORMAT(1H1,59X,12HENTRY FARFLD,/,/,22H SUBROUTINE INPUT DATA,/,13FFDL0036
      5H PLATE WIDTH=.E12.5,/,14H PLATE LENGTH=.E12.5,/,33H NUMBER OF ELEFFDL0037
      5HMENTS ACROSS WIDTH=.I5,/,17H PLATE THICKNESS=.E12.5,/,21H STIFFENEFDL0038
      3H THICKNESS=.E12.5,/,33H HOLE CENTER NODE REFERENCE CODE=.I5,/,27HFFDL0039
      3H HOLE ELEMENT INPUT RADIUS=.E12.5,/,26H APPLIED TENSION PRESSURE=.FFDL0040
      3HE12.5,/,31H STATIC CONDENSATION INDICATOR=.I4,37H (INDICATOR=0:FFDL0041
      3HCHESIRE CAT PROBLEM:/,39X,73H INDICATOR=1:PLATE WITH FNO NODES ANFFDL0042
      3H WITH HOLE-NO LOADS OR 3.C. APPLIED))
      3HWRITE(KW,6001) LGTH,F,NIDSS
      3HWRITE(KW,6001) LGTH,F,NIDSS
6001 FORMAT(23H SUBROUTINE STATISTICS:/,/,12H LUG LENGTH=.E12.5,/,13HMOFFDL0045
      3HLE OFFSET=.E12.5,/,75H NUMBER OF INTERNAL D.O.F.S TO RE CONDENSED FFDL0046
      3HOUT OF GLOBAL STIFFNESS MATRIX=.I5,/)
10 CALL SETUP(NSIZE,NCON,LMASTR,RSS,ISS)
      1HMASTR=IBEGIN(4)
      1HDOOF1=1
      1HDOOF2=2*(3*NDW+1)-1
      1HDO 30 N=1,2
      1HIPNTR=IMASTR*NET*(N-1)*4*NDW
      1HISS(IMASTR*N-1)=IPNTR
      1HIF(N.EQ.2) IDOF1=(2*NDW+1)*2-1
      1HIF(N.EQ.2) IDOF2=(NDW+1)*2-1
      1HMASTR=IDOF1+2*NDW-1
      1HDO 20 I=IDOF1,LMASTR
      1HISS(IPNTR)=I
20 IPNTR=IPNTR+1
      1HMASTR=IDOF2+2*NDW-1
      1HDO 30 I=IDOF2,LMASTR
      1HISS(IPNTR)=I
30 IPNTR=IPNTR+1
      1HISS(IMASTR*2)=IPNTR
      1HDOOF1=1
      1HDOOF2=(NDW+1)*2-1
      1HMASTR=NDW*2
      1HDO 40 I=IDOF1,LMASTR
      1HISS(IPNTR)=I
40 IPNTR=IPNTR+1

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LMASTR=4*NDW
DO 50 I=IDOF2,LMASTR
ISS(IPNTR)=I
IPNTR=IPNTR+1
IDOF1=R*NDW+1
IDOF2=IDOF1+47
DO 60 I=IDOF1,IDOF2
ISS(IPNTR)=I
IPNTR=IPNTR+1
IF(KT1.EQ.KW) GO TO AN
IDOF1=IREGIN(4)
IDOF2=IDOF1+2
WRITE(KW,6002) (ISS(I),I=IDOF1,IDOF2)
6002 FORMAT(22HMASTER ASSEMBLY LIST:*,10H POINTERS:*,3I5)
DO 70 N=1,2
IDOF1=IREGIN(4)*NET*(N-1)+4*NDW
IDOF2=IDOF1+4*NDW-1
WRITE(KW,6003) N
6003 FORMAT(13H01UG ELEMENT ,I2,9H 0.0.F.S:*/)
70 WRITE(KW,6004) (ISS(I),I=IDOF1,IDOF2)
6004 FORMAT(1H ,20I4)
IDOF1=IDOF2+1
IDOF2=IEND(4)
WRITE(KW,6005)
6005 FORMAT(23H0CZONE ELEMENT 0.0.F.S:*/)
WRITE(KW,6006) (ISS(I),I=IDOF1,IDOF2)
80 CALL ORK(NSIZE,RSS,ISS)
NIDSV=NIDSS
DO 90 I=1,2
ISAVE1(I)=ISZ(I)
DO 100 I=1,6
ISAVE2(I)=IBEGIN(I)
ISAVE3(I)=IEND(I)
CALL LUG(WIDTH,LENGTH,NELW,THK,STFTHK,C,NSZWRK,RWORK,IWORK)
DO 110 I=1,2
ISZ(I)=ISAVE1(I)
110

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DO 120 I=1,6
  IBEGIN(I)=ISAVE2(I)
  IEND(I)=ISAVE3(I)
DO 130 N=1,2
  CALL ASMSUB(N,RWORK,IWORK,RSS,ISS)
DO 140 I=1,2
  ISAVE1(I)=ISZ(I)
DO 150 I=1,6
  ISAVE2(I)=IBEGIN(I)
  ISAVE3(I)=IEND(I)
  CALL CZONE(WIDTH,NELW,THK,STFTHK,C,S,CNODE,RI,NSZWRK,RWORK,IWORK,
    RHOLE,IMOLF)
DO 160 I=1,2
  ISZ(I)=ISAVE1(I)
DO 170 I=1,6
  IBEGIN(I)=ISAVE2(I)
  IEND(I)=ISAVE3(I)
  IRESS(I)=IBGSS(I)
DO 180 I=1,3
  ISZSS(I)=ISZSS(I)
  CALL ASMSUB(3,RWORK,IWORK,RSS,ISS)
  MIDSS=MIDSV
  IF (INDCTR.EQ. 0) GO TO 190
C PLATE WITH END NODES AND HOLE-NO R.C. OR APPLIED LOADS
  I=1
  CALL STACON(I,MIDSS,RSS,ISS)
  GO TO 220
C CHESIRE CAT PROBLEM
C APPLY B.C.
  190 IDOF1=(2*NDW+1)*2
  IDOF2=6*NDW
  IMASTR=1
  LMASTR=IBEGIN(5)-1
  DC 200 I=IDOF1,IDOF2,2
  ISS(IMASTR)=I
  RSS(LMASTR+I)=0.0

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200  IMASTR=IMASTR+1
      IDOF1=(2*NDW+1*NDW/2)*2-1
      ISS(IMASTR)=IDOF1
      RSS(LMASTR*IDOF1)=0.0
      IDOF1=6*NDW+2
      IDOF2=8*NDW-2
      F= W*PRESS*THK/2.0
      DO 210 I=IDOF1,IDOF2*2
        RSS(LMASTR*I)=RSS(LMASTR*I)*F
210  RSS(LMASTR*I*2)=RSS(LMASTR*I*2)*F
      C ADD IN ADDITIONAL LOADS FOR STIFFENED EDGES
      F=(STFTHK-THK) *W*PRESS/2.0
      RSS(LMASTR*IDOF2*2)=RSS(LMASTR*IDOF2*2)*F
      RSS(LMASTR*IDOF2 )=RSS(LMASTR*IDOF2 )*F
      CALL BCON(RSS,ISS)
      I=1
      CALL STACON(I,MIDSS,RSS,ISS)
220  DO 221 I=1,2
221  ISZSS(I)=IS7(I)
      ISZSS(3)=NINSS
      DO 222 I=1,6
222  IRGSS(I)=IREGIN(I)
      IF( KTL .EQ. KW ) GO TO 230
      WRITE(KW,6006)
6006 FORMAT(1M0,59X,11HEXIT FARFLD.///)
230  CONTINUE
      RETURN
      END

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